GEOMETRIC DESIGN STANDARDS FOR RURAL PRINCIPAL ARTERIAL SYSTEM (GS-1M)

	TERRAIN	DESIGN SPEED (km/h)	MINIMUM RADIUS (METERS)	(6) STOPPING SIGHT DISTANCE (METERS)	MIN. WIDTH OF LANE	MINI WIDT TO SHOU	1) MUM IH OF TAL LDERS + Paved)	(2) PAVED SHOULDER WIDTH		(3) WIDTH OF DITCH (FRONT SLOPE)	(4) SLOPE	(5) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES
				MIN.		FILL	CUT	RT.	LT.			
	LEVEL	110	502	220								2 THRU LANES SAME DIRECTION =
FREEWAYS	ROLLING	100	394	185	3.6 m	5.2 m	4.3 m	3.6 m	1.2 m	3.6 m	CS-4B	1.5m + PAVE. WIDTH + 4.3m 3 OR MORE THRU LANES
	MOUNTAINOUS	80	230	130							CS-4E	SAME DIRECTION = 4.3m + PAVE. WIDTH + 4.3m
	LEVEL	110	502	220							CS-4 OR	
OTHER	LEVEL	100	394	185						3.0 m	4B	UNDIVIDED & DIVIDED 3 OR MORE THRU LANES
OTHER	POLLING	100	394	185	3.6 m	3.9 m	3.0 m	2.4 m	1.2 m		CS-4 OR	SAME DIRECTION = 3.0m + PAVE, WIDTH +3.0m
PRINCIPAL ROLLING ARTERIALS MOUNTAINOUS	ROLLING	80	230	130	3.6 m	3.9 111	3.0 111	2.4 111	1.2 111		4E	
	MOLINITAINOLIS	80	230	130						1.8 m	CD-3 OR	2 THRU LANES (DIVIDED) SAME DIRECTION
	60	124	85							3B	1.5m + PAVE. WIDTH + 3.0m	

GENERAL NOTES

<u>Freeways</u> - A design speed of 110 km/h should be used for Rural Freeways. Where terrain is mountainous a design speed of 100 km/h or 80 km/h, which is consistent with driver expectancy, may be used. All new and major reconstructed Interstate facilities will have a 110 km/h design speed unless a lower design speed is approved by the Location and Design Engineer and FHWA.

Other Principle Arterials - A design speed of 60 to 110 km/h should be used depending on terrain, driver expectancy and whether the design is constructed on new location or reconstruction of an existing facility. An important safety consideration in the selection of one of the lower design speeds in each range is to have a properly posted speed limit which is enforced during off peak hours.

Incorporated towns or other built-up areas, Urban Standard GS-5(M) may be used for design. "Built-up" is where there is sufficient development along the roadway that justifies a need to channelize traffic into and out of properties utilizing curb and gutter.

Standard TC-5.01R(M) (2001 AASHTO Green Book) superelevation based on 8% maximum is to be used for all Rural Principle Arterials.

RELATIONSHIP OF MAXIMUM GRADES TO DESIGN SPEEDS												
	FR	FREEWAYS ARTERIALS										
TYPE OF	O (km/h)										
TERRAIN	80 100 110 60 80					100	110					
		G	RADES	(PER	CENT)	*						
LEVEL	4	3	3	5	4	3	3					
ROLLING	5	4	4	6	5	4	4					
MOUNTAINOUS	6	6	5	8	7	6	5					

Grades 1 percent steeper than the value shown may be used on Rural Freeways in extreme situations for one-way downgrades except in mountainous terrain.

Clear Zone and Recoverable Area information can be found in Appendix A(M), Section A-2(M) of the Road Design Manual.

If medians are included, see Section 2E-3 of Chapter 2D of the $\underline{\text{Road}}$ $\underline{\text{Design Manual}}$

FOOTNOTES

- (1) Shoulder widths shown are for right shoulders and independently graded median shoulders with no additional width necessary for guardrail situation. On non-Interstate a 2.4 m graded median shoulder will be provided when the mainline is 4 lanes (both directions). For 6 or more lanes, the median shoulder provided will be the same as that shown for independent grading. On Freeways, if truck traffic is less than 250 DDHV, the minimum width of total shoulder should be 4.6 m for fills and 3.6 m for cuts.
- (2) When the mainline is 6 or more lanes, the left paved shoulder width should be the same as the right paved shoulder. On Freeways, if truck traffic is less than 250 DDHV, the minimum right paved shoulder width should be 3.0 m.
- (3) Ditch slopes to be 6:1 3.0 m and 3.6 m widths and 4:1 1.8 m width.
- (4) Additional or modified slope criteria to apply where shown on typical sections.
- (5) Vertical clearance at roadway underpasses for new and reconstructed bridges is to be 5.05 m (0.3 m additional clearance required for non-vehicular overpasses).
 - 4.2m shoulder may be reduced to 3.0m minimum when truck traffic is less than 250 DDHV.
- (6) For intersection sight distance requirements, see Appendix C, Table C-1-5.

FIGURE A - 1 - 1M*

^{*} Rev. 1/09

GEOMETRIC DESIGN STANDARDS FOR RURAL MINOR ARTERIAL SYSTEM GS-2M)

TRAFFIC VOLUME	TERRAIN	DESIGN SPEED (km/h)	MINIMUM RADIUS (METERS)	(8) STOPPING SIGHT DISTANCE (METERS)	(2) MIN. WIDTH OF LANE	MIN. V OF T SHOU (GRAI	3) VIDTH OTAL LDERS DED & /ED)	PA\ SHOL	4) /ED JLDER DTH	(5) WIDTH OF DITCH (FRONT SLOPE)	(6) SLOPE	(7) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL
				MIN.	LAINE	FILL W/GR	CUT & FILL	RT.	LT.	SLOPE)		CLEARANCES
	LEVEL	110	502	220							CS-4,	
(1)	LLVLL	100	394	185						3.0 m	CS-4A	3.0 m PLUS
ADT	ROLLING	100	394	185	3.6 m	3.9 m	3.0 m	2.4 m	1.2 m		OR CS- 4C	PAVEMENT WIDTH
OVER		80	230	130	0.01.1	0.0111	0.0111		1.2		40	PLUS 3.0 m
2000	MOUNTAINOUS	80	230	130						1.8 m	CS-3 OR	
		60	124	85							CS-3B	
	LEVEL	110	502	220	3.6 m				1.2 m		CS-4,	
(1)		100	394	185	3.6 m			1.8 m		1.8 m	CS-4A	
ADT 1500	ROLLING	80	394	185		3.3 m	2.4 m				OR CS- 4C	
TO 2000		80	230	130	3.3 m							
	MOUNTAINOUS	60	230 124	130 85	3.3 M						CS-3 OR CS-3B	2.4 m PLUS
		110	502	220								PAVEMENT WIDTH
	LEVEL	100	394	185	3.6 m						CS-4, CS-4A	PLUS 2.4 m
(1)		100	394	185							OR CS-	
ADT 400	ROLLING	80	230	130		3.3 m	2.4 m	1.8 m	1.2m	1.8 m	4C	
TO 1500		80	230	130	3.3 m						CS-3 0R	
	MOUNTAINOUS	60	124	85							CS-3B	
	15)/51	110	502	220							CS-4,	
CURRENT	LEVEL	100	394	185	3.6 m						CS-4A	
ADT	ROLLING	100	394	185		2.7 m	1.8 m	1.2 m	1.2 m	1.8 m	OR CS-	1.8 m PLUS PAVEMENT WIDTH
UNDER	KULLING	80	230	130		2./ m	1.8 m	1.2 m	1.2 m	1.8 m	4C	PAVEMENT WIDTH PLUS 1.8 m
400	MOLINITAINOLIS	80	230	130	3.3 m						CS-3 OR	1 200 1.0111
	MOUNTAINOUS -	60	124	85							CS-3B	

GENERAL NOTES

Rural Minor Arterials are designed with design speeds of 80 to 110 km/h, dependent on terrain features and traffic volumes, and occasionally may be as low as 60 km/h in mountainous terrain.

In incorporated towns or other built-up areas, Urban Standard GS-6(M) may be used for design. "Built-up" is where there is sufficient development along the roadway that justifies a need to channelize traffic into and out of properties utilizing curb and gutter.

Standard TC-5.01R(M) (2001 AASHTO Green Book) superelevation based on 8% maximum is to be used for Rural Minor Arterials.

If medians are included, see Section 2E of the Road Design Manual.

Clear zone and Recoverable Area information can be found in Appendix A(M), Section A-2(M) of the Road Design Manual.

RELATIONSHIP OF MAXIMUM GRADES TO DESIGN SPEEDS											
	DE	SIGN SF	PEED (kn	n/h)							
TYPE OF TERRAIN	60	80	100	110							
	GRADES (PERCENT)										
LEVEL	5	4	3	3							
ROLLING	6	5	4	4							
MOUNTAINOUS	8	7	6	5							

For Passing Sight Distance Criteria See Current AASHTO Green Book.

FOOTNOTES

- (1) Use Design Year ADT for new construction and reconstruction projects (not applicable to R.R.R. projects or roads with ADT < 400) in accordance with Road Design Manual, Chapter 2A, "REQUEST FOR TRAFFIC DATA" and Form LD-104.
- (2) Lane width to be 3.6 m at all interchange locations. For projects not on the National Highway System, width of traveled way may remain at 6.6 m on reconstructed highways where alignment and safety records are satisfactory.
- (3) If graded median is used, the width of median shoulder is to be 2.4 m.
- (4) The Paved widths shown are the widths to be used if the Materials Division recommends the shoulders be paved or stabilized. When the mainline is 4 lanes (both directions) a minimum 2.4 m wide paved shoulder will be provided on the right of traffic and a minimum 1.2 m wide paved shoulder on the median side. Where the mainline is 6 or more lanes, both right and median paved shoulders will be 2.4 m in width. If paved shoulders are not recommended by the Materials Division the mainline pavement structure will be extended 0.3 m at the same slope into the shoulder to eliminate raveling of the pavement edge.
- (5) Ditch slopes to be 6:1 3.0 m width, 4:1 1.8 m width.
- (6) Additional or modified slope criteria to be applied where shown on typical sections.
- (7) Vertical clearance at roadway underpasses for new and reconstructed bridges is to be 5.05 m (0.3 m additional clearance required for non-vehicular overpasses).
- (8) For intersection sight distance requirements, see Appendix C, Table C-1-5.

FIGURE A - 1 - 2M*

^{*} Rev. 1/09

GEOMETRIC DESIGN STANDARDS FOR RURAL COLLECTOR ROAD SYSTEM (GS-3M)

TRAFFIC VOLUME	TERRAIN	DESIGN SPEED (km/h)	MINIMUM RADIUS (METERS)	(9) STOPPING SIGHT DISTANCE (METERS MIN.	(2) MIN. WIDTH OF LANE	MIN. V OF GF	(4) VIDTH RADED LDERS CUT & FILL	(5) WIDTH OF DITCH (FRONT SLOPE)	(6) RECOMMENDED SLOPE	(7) (8) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES
(1)	LEVEL	LEVEL 100 394 185			20	CS-4, CS-4A,				
ADT	ROLLING	80	230	130				3.0 m	OR CS-4C	2.4 m PLUS
OVER 2000	MOUNTAINOUS	60	124	85	3.6 m	3.3 m	2.4 m	1.8 m	CS-3 OR CS-3B	PAVEMENT WIDTH PLUS 2.4m
(1)	LEVEL	80	230	130				1.8 m	CS-4, CS-4A,	
ADT	ROLLING	60	124	85				1.0111	OR CS-4C	1.2 m PLUS
1500 TO 2000	MOUNTAINOUS	50	83	65	3.3 m	2.7 m	1.8 m	1.2 m	CS-3 OR CS-3B	PAVEMENT WIDTH PLUS 1.2 m
(1)	LEVEL	80	230	130	3.3 m			1.8 m	CS-4, CS-4A,	
ADT 400	ROLLING	60	124	85	3.5 111			1.0111	OR CS-4C	1.0 m PLUS
TO 1500	MOUNTAINOUS	50	83	65	3.0 m	2.4 m	1.5 m	1.2 m	CS-3 OR CS-3B	PAVEMENT WIDTH PLUS 1.0 m
CURRENT	LEVEL	60	124	85				1.8 m		0.6 m PLUS
ADT	ROLLING	50	83	65	3.0 m	2.1 m	0.6 m	1.2 m	CS-1	PAVEMENT WIDTH
UNDER400	MOUNTAINOUS	30	29	35				1.2 111		PLUS 0.6 m

GENERAL NOTES

Geometric design features should be consistent with a design speed appropriate for the conditions.

Low design speeds (60 km/h and below) are generally applicable to highways with curvilinear alignment in rolling or mountainous terrain and where environmental conditions dictate.

High speed design (70 km/h and above) are generally applicable to highways in level terrain or where other environmental conditions are favorable.

Intermediate design speeds would be appropriate where terrain and other environmental conditions are a combination of those described for low and high speed.

The designer should strive for higher values than the minimum where conditions of safety dictate and costs can be supported.

In incorporated towns or other built-up areas, Urban Standard GS-7M may be used. "Built-up" is where there is sufficient development along the roadway that justifies a need to channelize traffic into and out of properties utilizing curb and gutter.

Standard TC-5.01R(M) (2001 AASHTO Green Book) superelevation based on 8% Maximum to be used for Rural Collectors.

Maximum grades of short length (less than 150 m), on one-way downgrades and on low-volume Rural Collectors may be 2 percent steeper.

T)/DE 05	DESIGN SPEED (km/h)									
TYPE OF TERRAIN	30	50	60	80	100	110				
		GRAD	ES (F	PERC	ENT)					
LEVEL	7	7	7	6	5	4				
ROLLING	10	9	8	7	6	5				
MOUNTAINOUS	12	10	10	9	8	6				

Clear zone and Recoverable Area information can be found in Appendix A(M), Section A-2(M) of the ${\hbox{\tt Road Design Manual}}.$

For Passing Sight Distance Criteria See Current AASHTO Green Book.

FOOTNOTES

- (1) Use Design Year ADT for new construction and reconstruction projects (not applicable to R.R.R. projects or roads with ADT < 400) in accordance with Road Design Manual, Chapter 2A, "REQUEST FOR TRAFFIC DATA" and Form LD-104.
- (2) Lane width to be 3.6 m at all interchange locations.
- (3) Provide 1.2 m wide paved shoulders when design year ADT exceeds 2000 VPD, with 5% or more truck and bus usage. All shoulders not being paved will have the mainline pavement structure extended 0.3 m on the same slope into the shoulder to eliminate raveling at the pavement edge.
- (4) When the mainline is four lanes with ADT >2000, a minimum paved shoulder width of 1.8 m right of traffic and 0.9 m left of traffic will be provided.
- (5) Ditch slopes to be 6:1 3.0 m width, 4:1 1.8 m width, 3:1 - 1.2 m width.
- (6) Additional or modified slope criteria to be applied where shown on typical sections.
- (7) Where the approach roadway width (traveled way plus shoulder) is surfaced, that surfaced width shall be carried across all structures if that width exceeds the width shown in this table.
- (8) Vertical clearance at roadway underpasses for new and reconstructed bridges is to be 5.05 m desirable and 4.45 m minimum (0.3 m additional clearance required for non-vehicular overpasses).
- (9) For intersection sight distance requirements, see Appendix C, Table C-1-5.

FIGURE A - 1 - 3M*

^{*} Rev. 1/09

GEOMETRIC DESIGN STANDARDS FOR RURAL LOCAL ROAD SYSTEM (GS-4M)

TRAFFIC VOLUME	TERRAIN	DESIGN SPEED (km/h)	MINIMUM RADIUS (METERS)	(9) STOPPING SIGHT DISTANCE	(2) MIN. WIDTH OF SURFACING OR	MIN. V	4)(5) VIDTH RADED LDERS	(6) WIDTH OF DITCH (FRONT	(7) RECOMMENDED SLOPE	(8) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS	
		(Idiliii)		MIN.	PAVEMENT	FILL W/GR	CUT & FILL	SLOPE)		AND VERTICAL CLEARANCE	
(1)	LEVEL	80	230	130				1.8 m	CS-4, 4A OR 4C	APPROACH	
ADT	ROLLING	60	124	85	7.2 m	3.3 m	2.4 m	1.0111	CS-3, 3A OR 3B	ROADWAY	
OVER 2000	MOUNTAINOUS	50	83	65				1.2 m	00 0, 0, 10, 10,	WIDTH	
(1)	LEVEL	80	230	130				1.8 m	CS-4, 4A OR 4C		
ADT 1500	ROLLING	60	124	85	6.6 m	2.7 m	1.8 m		CS-3, 3A OR 3B	10 m PLUS	
TO 2000	MOUNTAINOUS	50	83	65				1.2 m	00 0, 0/10/102	PAVEMENT WIDTH	
(1)	LEVEL	80	230	130	6.6 m			1.8 m	CS-4, 4A OR 4C	PLUS 10 m	
ADT 400	ROLLING	60	124	85	6.0 m	2.4 m	1.5 m	1.0111	CS-3, 3A OR 3B		
TO 1500	MOUNTAINOUS	50	83	65	0.0 111			1.2 m	00 0, 0/ C/ C/C	<u>l</u>	
CURRENT	LEVEL	60	124	85				1.8 m			
ADT 400	ROLLING	50	83	65	5.4 m	2.1 m	0.6 m	1.2 m	CS-1		
TO 250	MOUNTAINOUS	30	29	35				1.2111			
CURRENT	LEVEL	50	83	65						0.6 m PLUS	
ADT 250	ROLLING	50	83	3	5.4 m	2.1 m	0.6 m	1.2 m	CS-1	PAVEMENT WIDTH	
TO 50	MOUNTAINOUS	30	29	35						PLUS 0.6 m	
CURRENT	LEVEL	50	83	65							
ADT	ROLLING	30	29	35	5.4 m	2.1 m 0.6 m		1.2 m	CS-1		
UNDER 50	MOUNTAINOUS	30	29	50							

GENERAL NOTES

Low design speeds are generally applicable to roads with winding alignment in rolling or mountainous terrain where environmental conditions dictate.

High design speeds are generally applicable to roads in level terrain or where other environmental conditions are favorable.

Intermediate design speeds would be appropriate where terrain and other environmental conditions are a combination of those described for low and high speed.

Standard TC-5.01R(M) (2001 AASHTO Green Book) superelevation based on 8% maximum is to be used.

In incorporated towns or other built-up areas, Urban Standard GS-8(M) may be used. "Built-up" is where there is sufficient development along the roadway that justifies a need to channelize traffic into and out of properties utilizing curb and gutter.

For Passing Sight Distance Criteria See Current AASHTO Green Book

RELATIONSHIP OF MAXIMUM GRADES TO DESIGN SPEEDS												
	DESIGN SPEED (km/h)											
TYPE OF TERRAIN	30	50	60	80	100							
	GRADES (PERCENT)											
LEVEL	8	7	7	6	5							
ROLLING	11	10	10	8	6							
MOUNTAINOUS	16	14	13	10								

FOOTNOTES

- (1) Use Design Year ADT for new construction and reconstruction projects (not applicable to R.R.R. projects or roads with ADT < 400) in accordance with Road Design Manual, Chapter 2A, "REQUEST FOR TRAFFIC DATA" and Form LD-104.
- (2) Lane width to be 3.6 m at all interchange locations.
- (3) In mountainous terrain or sections with heavy earthwork, the graded width of shoulder in cuts may be decreased by 0.6 m, but in no case shall the shoulder width be less than 0.6 m.
- (4) Minimum shoulder slope shall be 8% on low side and same slope as pavement on high side (See St'd. GS-12).
- (5) Provide 1.2 m wide paved shoulders when design year ADT exceeds 2000 VPD, with 5% or more truck and bus usage. All shoulders not being paved will have the mainline pavement structure extended 0.3 m on the same slope into the shoulder to eliminate raveling at the pavement edge.
- (6) Ditch slopes to be 4:1 1.8 m width, 3:1 1.2 m width.
- (7) Additional or modified slope criteria to be applied where shown on typical sections.
- (8) Vertical clearance at roadway underpasses for new and reconstructed bridges is 5.05 m desirable and 4.45 m minimum (0.3 m additional clearance required for nonvehicular overpasses).
- (9) For intersection sight distance requirements, see Appendix C, Table C-1-5.

FIGURE A - 1 - 4M*

^{*} Rev. 1/09

GEOMETRIC DESIGN STANDARDS FOR URBAN PRINCIPAL ARTERIAL SYSTEM (GS-5M)

	DESIGN SPEED (k/m/h)	RAC	MUM DIUS ULS	(13) STOPPING SIGHT DISTANCE MIN.	MIN. WIDTH OF LANE	WID	MUM TH TAL LDERS	SHOU	VED ILDER ITH	(3) WIDTH OR DITCH (FRONT SLOPE)	(4) SLOPE	(7) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES
	110	502	-	220							CS-4 OR	2 THRU LANES SAME DIRECTION -
FREEWAYS	100	394	-	185	3.6m	6m 4.5m 3.6		3.0m	1.2m	3.6m	CS-4B	1.8m + PAVE. WIDTH + 3.6m 3 OR MORE THRU LANES
	80	230	-	130							CS-4 OR 4E	SAME DIRECTION - 4.2m + PAVE. WIDTH + 4.2m
OTHER	100	394	-	185	(12)					3.0m	CS-4	UNDIVIDED & DIVIDED 3 OR MORE THRU LANES
PRINCIPAL ARTERIAL WITH	80	280	-	130	3.6m	3.9m 3.0r		2.4m	1.2m		OR CS-4E	SAME DIRECTION = 3.0m + PAVE, WIDTH + 3.0m
SHOULDER	60	150	149	85	(5) (6) (12)					1.8m	CS-3	2 THRU LANES (DIVIDED) SAME DIRECTION -
DESIGN	50	99	94	65	3.3m						OR CS-3B	1.8m + PAVE. WIDTH + 3.0m
	DESIGN SPEED (km/h)	RAI	MUM DIUS	STOPPING SIGHT DISTANCE	MIN. WIDTH OF LANE	(8 STANI CURI GUT (14	DARD B & TER		FER RIP OTH	(9) MINIMUM SIDEWALK WIDTH	(10) SLOPE	NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES
OTHER	100	394	-	185								
PRINCIPAL	80	280	-	130	(12)	CG-7	l					
ARTERIAL WITH	70	215	227	105	3.6m		•	(11)		1.5m	2: 1	SAME AS CURB TO CURB OF APPROACHES
CURB &	60	150	149	85	(5) (6) (12)		-				OF APPROACHES	
GUTTER	50	99	94	65	3.5m	cc-e						

GENERAL NOTES

Freeways - Urban Freeways should accommodate desired safe operating speeds during non-peak hours, but should not be so high as to exceed the limits of prudent construction, right of way and socioeconomic costs due to the large proportion of vehicles which are accommodated during periods of peak flow when lower speeds are necessary. The design speeds for Freeways should never be less than 80 km/h.

On many Urban Freeways, particularly in suburban areas, a design speed of 100 km/h or higher can be provided with little additional cost above that required for 80 km/h design speed. The corridor of the mainline may be relatively straight and the character and location of interchanges may permit high speed design. Under these conditions, a design speed of 110 km/h is most desirable because the higher design speeds are closely related to the overall quality and safety of the facility.

Other Principal Arterials - Design speeds for Urban Arterials generally range from 60 to 100 km/h, and occasionally may be as low as 50 km/h. The lower (60 km/h and below) speeds apply in the central business district and intermediate areas. The higher speeds are more applicable to the outlying business and developing areas

Standard TC-5.01R (M) (Rural) (2001 AASHTO Green Book) superelevation based on 8% maximum is to be used for all Freeways and is to be used for all other Principal Arterials with a design speed

* Grades 1 percent steeper that the value shown may be used on Urban Freeways for extreme cases in urban areas where development precludes the use of flatter grades and for one-way downgrades, except in mountainous terrain.

RELATIONSHIP OF M	ИХАIМ	IUM (GRAD	ES T	O DE	ESIGN	I SPE	EEDS			
	FRE	FREEWAYS* ARTERIALS									
TYPE OF		DE	SIGN	SPE	ED (km/h	1)				
TERRAIN	80	100	110	50	60	70	80	100			
		GRADES (PERCENT)									
LEVEL	4	3	3	8	7	6	6	5			
ROLLING	5	4	4	9	8	7	7	6			
MOUNTAINOUS	6	6	5	11	10	9	9	8			

Standard TC-5.01U (M) (Urban) (2001 AASHTO Green Book) superelevation based on 4% maximum is to be used on Other Principal Arterials with a design speed less than 100 km/h.

Standard TC-5.04ULS (M) (Urban Low Speed) (2004 AASHTO Green Book) superelevation based on 2% maximum is to be used on Other Principal Arterials with a design speed less than or equal to 70 km/h. Clear Zone and Recoverable Area information can be found in Appendix A (M), Section A-2(M) of the Road Design Manual.

If medians are included, see Section 2E-3 of Chapter 2E of the Road Design Manual

A minimum 9.2 m width of surfacing or a minimum 9.2 m face to face of curb is to be used within incorporated cities or towns to qualify for maintenance payments.

For guidelines on Interchange Ramp, see Standard GS-R(M).

- FOOTNOTES
 Shoulder widths shown are for right shoulders and independently graded median shoulders. A 2.4m graded median shoulder will be provided when the mainline is 4 lanes (both directions). For 6 or more lanes, the median shoulder provided will be the same as that shown for independent grading. On Freeways, if truck traffic exceeds 250 DDHV, the minimum width of graded shoulder should be 5.1m for fills and 4.2m for cuts.
- (2) When the mainline is 6 or more lanes, the left paved shoulder width should be the same as the right paved shoulder. On Freeways, if truck traffic exceeds 250 DDHV, the right paved shoulder width should be 3.6 m, and on 6 or more lane Freeways, the left paved shoulder width should also be 3.6 m if truck traffic exceeds 250 DDHV.
- Ditch slopes to be 6:1 3.0 m and 3.6 m widths and 4:1 1.8 m width.
- (4) Additional or modified slope criteria to apply where shown on typical sections.
- Minimum lane width to be 3.6 m at all interchange locations.
- If heavy truck traffic is anticipated, an additional 0.3 m width is desirable. (6)
- Vertical clearance at roadway underpasses for new and reconstructed bridges is to be 5.05 m (0.3 m additional clearance required for non-vehicular overpasses). 4.2m shoulder may be reduced to 3.0m minimum when truck traffic is less than 250 DDHV.
- Or equivalent City or Town design.
- Width of 2.4 m or more may be needed in commercial areas.
- 3:1 and flatter slopes may be used when the right of way is behind the sidewalk (or sidewalk space) in residential or other areas where slopes will be maintained by the property owner.
- If a buffer strip is used between the back of curb and sidewalk, it should be $0.6\ m$ minimum.
- Situations having restrictions on trucks may allow the use of lanes 0.3m less in width.
- For intersection sight distance requirements, see Append., Table C-1-5
- Because Urban Principal Arterials are typically free-flowing, with higher operating speeds, Standard CG-7 is recommended for design speeds \geq 70 km/h. See current AASHTO "Green Book", Chapter 2.

FIGURE A - 1 - 5M

Rev. 1/09

GEOMETRIC DESIGN STANDARDS - URBAN MINOR ARTERIAL STREET SYSTEM (GS-6M)

	DESIGN SPEED (km/h)	RAC		(12) STOPPING SIGHT DISTANCE	(11) MIN. WIDTH OF LANE	STAN CUR GUT	(3) STANDARD CURB & GUTTER (10)		FER RIP OTH	(4) MINIMUM SIDEWALK WIDTH	(5) SLOPE	(6) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES		
	100	394	1	185		CG	- 7							
STREETS	80	280	-	130	3.6m		•	1				SAME AS CUBB TO CUBB		
CURB &	70	215	227	105				(10)		1.5m	2: 1	SAME AS CURB TO CURE OF APPROACHES		
GUTTER	60	150	149	85	(1) (2)									
	50	99	94	65	3.3m									
	DESIGN SPEED (km/h)	RAD	MUM DIUS ULS	STOPPING SIGHT DISTANCE MIN.	MIN. WIDTH OF LANE	MINI	CUT		VED ILDER ITH	(9) WIDTH OR DITCH (FRONT SLOPE)	(5) SLOPE	NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES		
STREETS	100	394	-	185						3.0m		3.0m + PAVEMENT		
WITH	80	280	1	130	3.6m	3.9m	3.0m	2.4m	1.2m		2: 1	WIDTH + 3.0m		
DESIGN	60	150	149	85	(1) (2)	3 3~	2 4 m	m 1.8m 1.2m		1.8m		2.4m + PAVEMENT		
	50	99	94	65	3.3m	3.3m 2.4m		ו.8m 1.2m				WIDTH + 2.4m		

GENERAL NOTES

Design Speeds for Urban Arterials generally range from 60 to 80 km/h and occasionally may be as low as 50 km/h. The lower (60 km/h and below) speeds apply in the central business district and intermediate areas. The higher speeds are more applicable to the outlying business and developing areas.

Standard TC-5.01R (M) (2001 AASHTO Green Book) superelevation based on 8% maximum is to be used for 100 km/h design speed.

Standard TC-5.01U (M) (Urban) (2001 AASHTO Green Book) superelevation based on 4% maximum is to be used for design speeds less than 100 km/h.

Standard TC-5.04ULS (M) (Urban Low Speed) (2004 AASHTO Green Book) superelevation based on 2% maximum may be used for design speeds less than or equal to 70 km/h.

Clear Zone and Recoverable Area information can be found in Appendix A (M), Section A-2 (M) of the $\underline{\text{Road Design Manual}}$.

If medians are included, see Section 2E-3 of Chapter 2E of the Road Design Manual.

A minimum 9.2 m width of surfacing or a minimum 9.2 m face to face of curb is to be used within incorporated cities or towns to qualify for maintenance payments.

RELATIONSHIP OF I	MUMIXAN	GRADE	S TO I	DESIGN	SPEEDS						
TV05 05		DESIGN	SPEED	(km/h)	1						
TYPE OF TERRAIN	50	60	70	80	100						
		GRADES (PERCENT)									
LEVEL	8	7	6	6	5						
ROLLING	9	8	7	7	6						
MOUNTAINOUS	11	10	9	9	8						

FOOTNOTES

- Lane width to be 3.6 m at all interchanges or if design year ADT exceeds 2000.
- (2) If heavy truck traffic is anticipated, an additional 0.3 m width is desirable.
- Or equivalent City or Town design.
- (4) A width of 2.4 m or more may be needed in commercial areas
- (5) 3:1 and flatter slopes may be used when the right of way is behind the sidewalk (or sidewalk space) in residential or other areas where slopes will be maintained by the property owner.
- (6) Vertical clearance at roadway underpasses for new and reconstructed bridges is to be 5.05 m (0.3 m additional clearance required for non-vehicular overpasses).
- (7) If graded median is used, the width of median shoulder is to be 2.4 m (See Standard GS-11 for shoulder design).
- (8) The Paved widths shown are the widths to be used if the Materials Division recommends the shoulders be paved or stabilized. When the mainline is 4 lanes (both directions) a minimum 2.4 m wide paved shoulder will be provided on the right of traffic and a minimum 1.2 m wide paved shoulder on the median side. Where the mainline is 6 or more lanes, both the right and median paved shoulders will be 2.4 m in width. If paved shoulders are not recommended by the Materials Division, the mainline pavement structure will be extended 0.3 m at the same slope into the shoulder to eliminate raveling of the pavement edge.
- (9) Ditch slope to be 6:1 3.0 m width and 4:1 1.8 m width.
- (10) If a buffer strip is used between the back of curb and sidewalk, it should be 0.6 m minimum.
- (11) Situations having restrictions on trucks may allow the use of lanes 0.3 m less in width.
- (12) For intersection sight distance requirements, see Appendix C, Table C-1-5.

FIGURE A - 1 - 6M*

^{*} Rev. 1/09

GEOMETRIC DESIGN STANDARDS FOR URBAN COLLECTOR STREET SYSTEM (GS-7M)

	DESIGN SPEED (km/h)	RAD	MUM DIUS ULS	(11) STOPPING SIGHT DISTANCE	MIN. WIDTH OF LANE	(3) STANDARD CURB & GUTTER (10)	BUFFER STRIP WIDTH	(4) MINIMUM SIDEWALK WIDTH	(5) SLOPE	(8) (9) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES		
STREETS	80	280		130	3.6m	CG-7						
WITH	70	215	227	105	3.0111		(10)	1.5m	2: 1	SAME AS CURB TO CURB OF APPROACHES		
CURB & GUTTER	60	150	149	85	(1) (2)	CG-6						
GUITER	50	99	94	65	3.3m			<u> </u>				
	DESIGN SPEED (km/h)	RAC	MUM DIUS ULS	STOPPING SIGHT DISTANCE	MIN. WIDTH OF LANE	MINIMUM GRAE SHOUL FILL W/GR	WIDTH DED	(10) WIDTH OR DITCH (FRONT SLOPE)	(5) SLOPE	(8) (9) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES		
STREETS WITH	80	280	-	130	3.6m	3.3m	2.4m	1.8m		2.4m + PAVEMENT		
SHOULDER	60	150	149	85	(1) (2)				2: 1	WIDTH + 2.4m		
DESIGN	50	99	94	65	3.3m	2.1m	1.2m	1.2m		1.2m + PAVEMENT WIDTH + 1.2m		

GENERAL NOTES

A minimum design speed of 50 km/h or higher should be used for collector streets, depending on available right of way, terrain, adjacent development and other area controls.

In the typical street grid, the closely spaced intersections usually limit vehicular speeds and thus make the effect of design speed of less significance. Nevertheless, the longer sight distances and curve radii commensurate with design speeds higher than the value indicated result in safer highways and should be used to the extent practicable.

Standard TC-5.01U (M) (Urban) (2001 AASHTO Green Book) superelevation based on 4% maximum.

Standard TC-5.04ULS (M) (Urban-Low Speed) (2004 AASHTO Green Book) superelevation based on 2% maximum may be used with a design speed of 70 km/h or less.

A minimum 9.2 m width of surfacing or a minimum 9.2 m curb to curb is to be used within incorporated cities or towns to qualify for maintenance payments.

Clear zone and Recoverable Area information can be found in Appendix A (M), Section A-2(M) of the Road Design Manual.

Maximum grades of short lengths (less than 150 m) and one-way down grades may be 2% steeper.

RELATIONSHIP OF M	MAXIMUM	GRADES T	O DESIGN	SPEEDS			
	DESIGN SPEED (MPH)						
TYPE OF TERRAIN	30	40 45		50			
	GRADES (PERCENT)						
LEVEL	9	9	8	7			
ROLLING	11	10	9	8			
MOUNTAINOUS	12	12	11	10			

FIGURE A - 1 - 7M*

FOOTNOTES

- (1) 3.6m when Design year ADT exceeds 2000. Where feasible, lanes should be 3.6m in industrial areas; however, where available or attainable R/W imposes severe limitations 3.3m lanes can be used in industrial areas., 3.0m lanes can be used in residential areas, based upon design speed and traffic volumes. (See AASHTO Green Bk., Exhibit 6-5).
- (2) Lane width to be 3.6 m at all interchange locations.
- (3) Or equivalent City or Town Design.
- (4) A width of 2.4 m or more may be needed in commercial areas.
- (5) 3:1 and flatter slopes may be used when right of way is behind the sidewalk (or sidewalk space) in residential or other areas where the slopes will be maintained by the property owner.
- (6) Ditch slopes to be 4:1 1.8 m width and 3:1 1.2 m width.
- (7) When Design year ADT exceeds 2000VPD, with greater than 5% total truck and bus usage: Provide 1.2 m wide paved shoulders when the graded shoulder is 1.5 m wide or greater or provide 1 m wide paved shoulders when the graded shoulder is 1.2 m wide. All shoulders not being paved will have the mainline pavement structure extended 0.3 m, on the same slope, into the shoulder to eliminate raveling at the pavement edge (See Standard GS-11 for shoulder design).
- (8) Where the approach roadway width (traveled way plus shoulder) is surfaced, that surfaced width shall be carried across all structures if that width exceeds the width shown in this table.
- (9) Vertical clearance at roadway underpasses for new and reconstructed bridges is to be 5.05 m desirable and 4.45 m minimum (0.3 m additional clearance required for non-vehicular overpasses).
- (10) If a buffer strip is used between the back of curb and sidewalk, it should be 0.6 m minimum.
- (11) For intersection sight distance requirements, see Appendix C, Table C-1-5.

^{*} Rev. 1/09

GEOMETRIC DESIGN STANDARDS FOR URBAN LOCAL STREET SYSTEM (GS-8M)

	DESIGN SPEED (km/h)	MINIM RAD (METI	IUS	(1) MAX. PERCENT OF GRADE	(11) STOPPING SIGHT DISTANCE (METERS)	(2) MIN. WIDTH OF LANE	(3) STANDARD CURB & GUTTER	(4) BUFFER STRIP WIDTH	(5) MIN. SIDEWALK WIDTH	(6) SLOPE	(9) (10) NEW AND RECONSTRUCTED MINIMUM BRIDGE WIDTHS AND VERTICAL CLEARANCES
STREETS WITH CURB	50	99	94	15	58m	3.0m	CG-6	(10)	1.5m	2:1	SAME AS CURB TO CURB OF
& GUTTER	30	34	24	10	30m	3.0111	000	(10)	1.5	,	APPROACHES
	DESIGN SPEED (km/h)		ium Ius Ers)	(1) MAX. PERCENT	STOPPING SIGHT	(2) MIN. WIDTH	(7) MIN. WIDTH GRADED SHOULDERS		(8) WIDTH OF DITCH	SLOPE	(9) NEW AND RECONSTRUCTED MINIMUM
	,	J	ULS	OF GRADE	(METERS)	OF LANE	FILL W/GR.	CUT & FILL	(FRONT) SLOPE		BRIDGE WIDTHS AND VERTICAL CLEARANCES
STREETS WITH	50	99	94	15	58m	3.0m	2.1 m	1.2 m	1.2 m	3:1	1.2 m + PAVEMENT
SHOULDER DESIGN	30	34	24]	30m]			1.2 111	0.1	WIDTH +1.2 m

GENERAL NOTES

Design Speeds is not a major factor for local streets. For consistency in design elements, design speeds ranging from 30 to 50 km/h may be used, depending on available right of way, terrain, adjacent development and other area controls.

In the typical street grid, the closely spaced intersections usually limit vehicular speeds, making the effect of a design speed of less significance.

Design speeds exceeding 50 km/h in residential areas may require longer sight distances and increased curve radii, which would be contrary to the basic function of a local street.

Standard TC-5.01U (M) (Urban) (2001 AASHTO Green Book) superelevation based on 4% maximum.

Standard TC-5.04ULS (M) (Urban Low Speed) (2004 AASHTO Green Book) superelevation based on 2% maximum may be used with a design speed of 70 km/h or less.

A minimum 9.2 m width of surfacing or a minimum 9.2 m curb to curb is to be used within incorporated cities or towns to qualify for maintenance payments.

FOOTNOTES

- (1) Grades in commercial and industrial areas should be less than 8 percent; desirably, less than 5 percent.
- (2) Where feasible, lanes should be 3.3 m wide and in industrial areas should be 3.6 m wide; however, where available or attainable right of way imposes severe limitations, 2.7 m lanes can be used in residential areas and 3.3 m lanes can be used in industrial areas.
- (3) Or equivalent City or Town design.
- (4) The minimum buffer strip width with no sidewalk or sidewalk space is to be 1.5 m.
- (5) Widths of 2.4 m or more may be needed in commercial areas.
- (6) 3:1 and flatter slopes may be used when the right of way is behind the sidewalk (or sidewalk space) in residential or other areas where slopes will be maintained by the property owner.
- (7) When Design year ADT exceeds 2000 VPD, with greater than 5% total truck and bus usage: Provide 1.2 m wide paved shoulders when the graded shoulder is 1.5 m wide or greater or provide 1 m wide paved shoulders when the graded shoulder is 1.2 m wide. All shoulders not being paved will have the mainline pavement structure extended 0.3 m, on the same slope, into the shoulder to eliminate raveling at the pavement edge (See Standard GS-12 for shoulder design).
- (8) Ditch slopes to be 3:1 1.2 m width.
- (9) Vertical clearance at roadway underpasses for new and reconstructed bridges is to be 5.05 m desirable and 4.45 m minimum (0.3 m additional clearance required for non-vehicular overpasses).
- (10) If a buffer strip is used between the back of curb and sidewalk, it should be 0.6 m minimum.
- (11) For intersection sight distance requirements, see Appendix C, Table C-1-5.

FIGURE A - 1 - 8M*

^{*} Rev. 1/09

GEOMETRIC DESIGN STANDARDS FOR SERVICE ROADS (GS-9M)

(1) DEAD END SERVICE ROADS UNDER 25 VPD										
PROPERTIES	DESIGN SPEED	MINIMUM RADIUS	STOPPING SIGHT	(2) MINIMUM TRAVELED	MINIMUM WIDTH OF SHOULDER		(3) WIDTH OF	SLOPES		
SERVED	(km/h)	(METERS)	DISTANCE (METERS)	WAY WIDTH	FILL W/GR.	CUT & FILL	DITCH (FRONT SLOPE)	020, 20		
1	20	10	40	3.6m	1.2m	0.6m	0.9m	(4)		
OVER 1	30	29	70	4.2m	1.5m	0.0111	0.3111	(+)		

GENERAL NOTES

The minimum design speed for service roads should be 30 km/h except for one lane service roads serving one property which may have a minimum design speed of 20 km/h.

Standard TC-5.01R (M) superelevation based on 8% maximum to be used (See 2001 AASHTO "Green Book").

For Passing Sight Distance Criteria See Current AASHTO Green Book.

RELATIONSHIP OF MAXIMUM GRADES TO DESIGN SPEEDS								
TVDE OF	DESIGN SPEED (km/h)							
TYPE OF TERRAIN	20 30 50 6							
	GRADES (PERCENT)							
LEVEL	8	8	7	7				
ROLLING	12	11	10	9				
MOUNTAINOUS	18	16	14	12				

FOOTNOTES

- (1) For through service roads and dead end service roads with over 25 VPD, use Standards shown for Local Roads and Streets (See Standard GS-12).
- (2) Under adverse conditions, intermittent shoulder sections or turnouts for passing may be required (see page 411, 2004 AASHTO "Green Book").
- (3) Ditch slope to be 3:1.
- (4) Slopes to be same as mainline when service road is parallel to or otherwise visible from the mainline. For other cases slopes should be in accordance with standards for Local Roads and Streets.

FIGURE A - 1 - 9M*

^{*} Rev. 1/09