

COMMONWEALTH of VIRGINIA

DEPARTMENT OF TRANSPORTATION 1401 EAST BROAD STREET RICHMOND, VIRGINIA 23219-2000

David S. Ekern, P.E. COMMISSIONER

October 28, 2009

MEMORANDUM

To: All Holders of the Virginia Department of Transportation's 2008 Road and Bridge Standards

The following is a list of sheets contained in the 2008 <u>Road and Bridge Standards</u> that have been revised. Please add these pages to your copy of the standards. An interim standard sheet will <u>not</u> be required in plan assemblies for the following sheets only. Changes to these sheets will not affect the basis of payment or estimates.

PAGE	REVISION
104.15	Clarified curb dimension
104.35	Clarified curb note
107.02	Clarified dimension location
300.01	Revised table of contents to reflect new pages
1001.01	Revised specification reference to 2007
1002.01	Revised specification reference to 2007
1003.02	Revised specification reference to 2007
1004.02	Revised specification reference to 2007
1004.22	Revised title of page
1004.23	Revised title of page
1005.02	Revised specification reference to 2007
1006.02	Revised specification reference to 2007
1006.09	Revised specification reference to 2007
1006.16	Revised specification reference to 2007
1006.23	Revised specification reference to 2007

The following is a list of revised standards to the 2008 Road and Bridge Standards that require an interim standard sheet to be in included in your plan assembly until the next edition of the imperial standards is published. Please add these pages to your copy of the standards. The respective interim standard sheet number has been placed with the revised standard. An interim standard sheet is available for each of these revised standards. The interim standard sheets are available on VDOT's web site, on the FTP server, and in Falcon DMS for VDOT personnel. These interim standard sheets will be required in plan assemblies for projects advertised May 11, 2010 and later.

PAGE	INTERIM	STANDARD	REVISION
203.05	IIS02_01	CG-12	REVISED BUFFER STRIP WIDTH
203.06	IIS02_02	CG-12	REVISED BUFFER STRIP WIDTH
203.07	IIS02_03	CG-12	REVISED BUFFER STRIP WIDTH
203.08	IIS02_04	CG-12	REVISED BUFFER STRIP WIDTH
203.08	A IIS02_05	CG-12	CLARIFIED SECTIONS TO MEET ADA
304.01	IIS03_03	RS-1	REVISED NOTES
304.02	IIS03_04	RS-3	REVISED NOTES
304.03	IIS03_05	RS-4	NEW STANDARD
304.04	IIS03_06	RS-5	NEW STANDARD
1005.1	7 IIS10_01	BCQ-30	REVISED CONCRETE QUANTITY

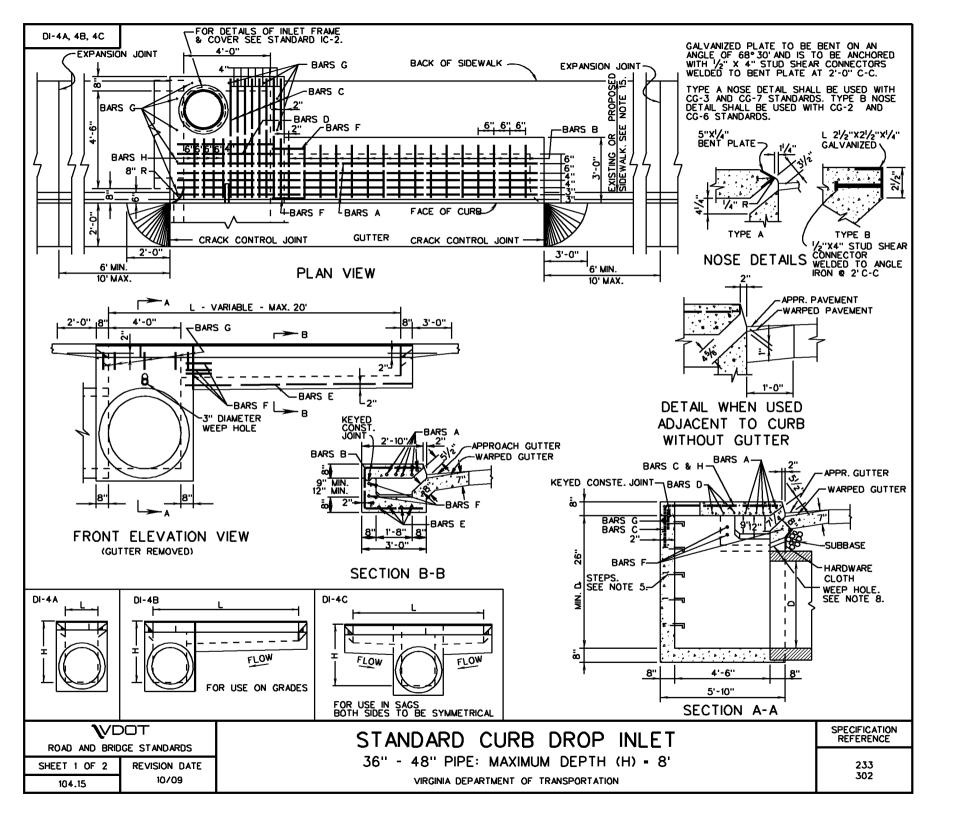
If you have any questions or comments regarding this revision to the publication, please contact Chuck Patterson, at (804) 786-1805, of the Standards and Special Design Section.

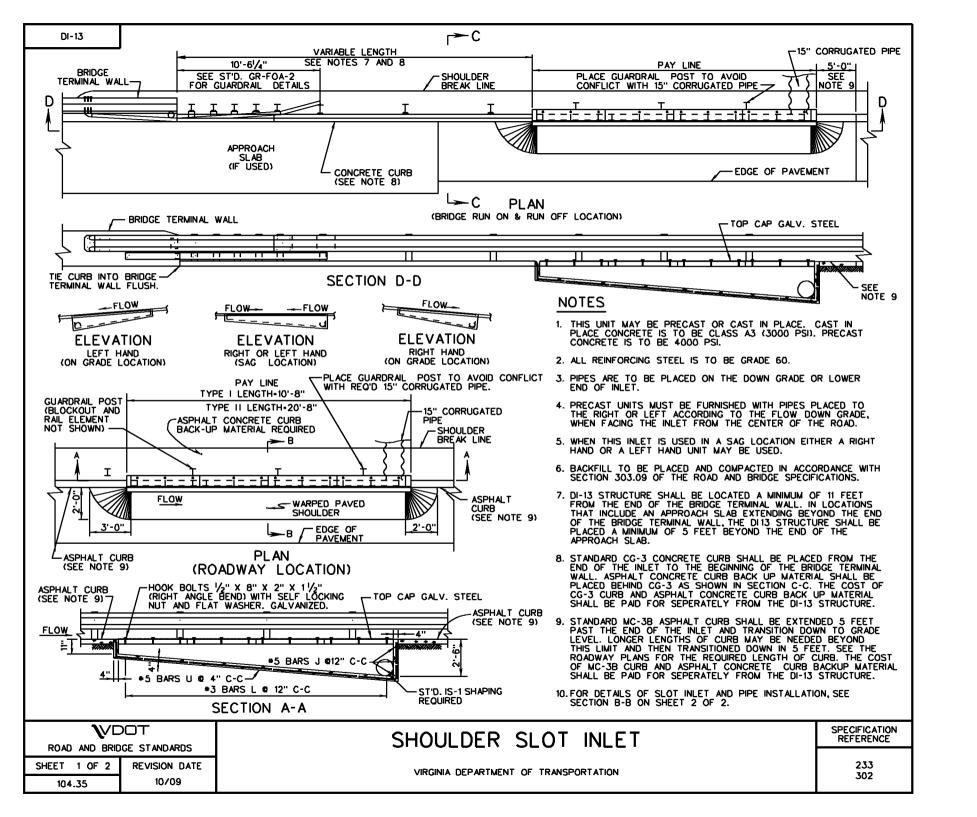
Sincerely,

Signature on file: October 28, 2009

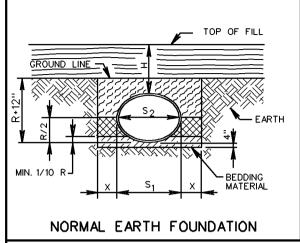
Mohammad Mirshahi, P.E.

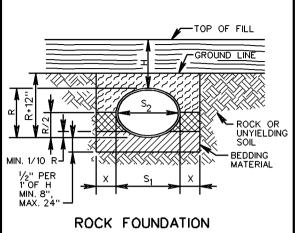
State Location and Design Engineer

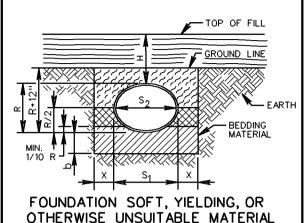




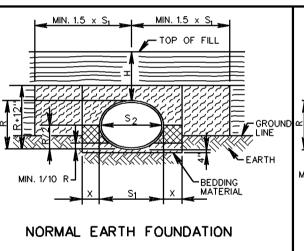
NO PROJECTION OF PIPE ABOVE GROUND LINE

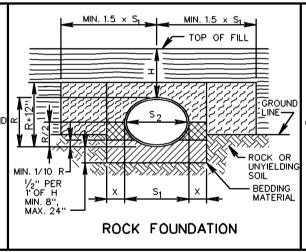


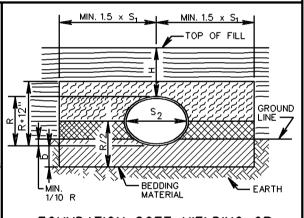




PIPE PROJECTION ABOVE GROUND LINE







FOUNDATION SOFT, YIELDING, OR OTHERWISE UNSUITABLE MATERIAL



BEDDING MATERIAL IN ACCORDANCE WITH SECTION 302 OF THE ROAD AND BRIDGE SPECIFICATIONS.



CLASS I BACKFILL MATERIAL IN ACCORDANCE WITH SECTION 302 OF THE ROAD AND BRIDGE SPECIFICATIONS.



REGULAR BACKFILL MATERIAL IN ACCORDANCE WITH SECTION 302 OF THE ROAD AND BRIDGE SPECIFICATIONS.



EMBANKMENT

NOTES:

FOR GENERAL NOTES ON PIPE BEDDING, SEE INSTALLATION OF PIPE CULVERTS AND STORM SEWERS GENERAL NOTES ON SHEET 107.00.

CRUSHED GLASS CONFORMING TO THE SIZE REQUIREMENTS FOR CRUSHER RUN AGGREGATE SIZE 25 AND 26 MAY BE USED IN PLACE OF CLASS I BACKFILL.

SPECIFICATION REFERENCE

302 303 INSTAL. OF PIPE CULVERTS AND STORM SEWERS ELLIP. PIPE BEDDING AND BACKFILL - METHOD "A"

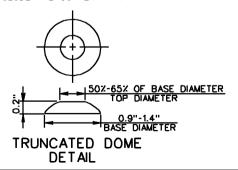
VIRGINIA DEPARTMENT OF TRANSPORTATION

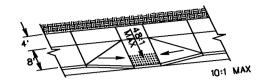
V DOT

ROAD AND BRIDGE STANDARDS

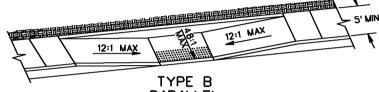
REVISION DATE 10/09 107.02

- THE DETECTABLE WARNING SHALL BE PROVIDED BY TRUNCATED DOMES.
- DETECTABLE WARNING TO BE CLASS A-3 CONCRETE (CLASS A-4 IF PRECAST) WITH SLIP RESISTANT INTEGRAL SURFACE COVERING THE FULL WIDTH OF THE RAMP FLOOR BY 2 FOOT IN LENGTH IN THE DIRECTION OF PEDESTRIAN TRAVEL. OTHER TYPES OF MATERIAL WITH THE TRUNCATED DOMES DETECTABLE WARNING MAY BE USED WITH THE APPROVAL OF THE ENGINEER.
- SLOPING SIDES OF CURB RAMP MAY BE POURED MONOLITHICALLY WITH RAMP FLOOR OR BY USING PERMISSIBLE CONSTRUCTION JOINT WITH REQUIRED BARS.
- IF RAMP FLOOR IS PRECAST, HOLES MUST BE PROVIDED FOR DOWEL BARS SO THAT ADJOINING FLARED SIDES CAN BE CAST IN PLACE AFTER PLACEMENT OF PRECAST RAMP FLOOR, PRECAST CONCRETE SHALL BE CLASS A-4.
- REQUIRED BARS ARE TO BE NO. 5 X 8" PLACED 1 CENTER TO CENTER ALONG BOTH SIDES OF THE RAMP FLOOR, MID-DEPTH OF RAMP FLOOR. MINIMUM CONCRETE COVER $1\frac{1}{2}$ ".
- CURB / CURB AND GUTTER SLOPE TRANSITIONS ADJACENT TO CURB RAMPS ARE INCLUDED IN PAYMENT FOR CURB / CURB AND GUTTER.
- CURB RAMPS ARE TO BE LOCATED AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER. THEY ARE TO BE PROVIDED AT INTERSECTIONS WHEREVER AN ACCESSIBLE ROUTE WITHIN THE RIGHT OF WAY OF A HIGHWAY FACILITY CROSSES A CURB REGARDLESS OF WHETHER SIDEWALK IS EXISTING, PROPOSED, OR NONEXISTENT. THEY MUST BE LOCATED WITHIN PEDESTRIAN CROSSWALKS AS SHOWN ON PLANS OR AS DIRECTED BY THE ENGINEER, AND SHOULD NOT BE LOCATED BEHIND VEHICLE STOP LINES, EXISTING LIGHT POLES, FIRE HYDRANTS, DROP INLETS, ETC. ACCESSIBLE ROUTES PROVIDE A CONTINUOUS UNOBSTRUCTED, STABLE, FIRM AND SLIP RESISTANT PATH CONNECTING ALL ACCESSIBLE ELEMENTS OF A FACILITY THAT CAN BE APPROACHED, ENTERED AND USED BY PEDESTRIANS.
- RAMPS MAY BE PLACED ON RADIAL OR TANGENTIAL SECTIONS PROVIDED THAT THE CURB OPENING IS PLACED WITHIN THE LIMITS OF THE CROSSWALK AND THAT THE SLOPE AT THE CONNECTION OF THE CURB OPENING IS PERPENDICULAR TO THE CURB.
- TYPICAL CONCRETE SIDEWALK IS 4" THICK, WHEN THE ENTRANCE RADII CANNOT ACCOMMODATE THE TURNING REQUIREMENTS OF ANTICIPATED HEAVY TRUCK TRAFFIC, REFER TO STANDARD CG-13, COMMERCIAL ENTRANCE (HEAVY TRUCK TRAFFIC) FOR CONCRETE DEPTH.
- WHEN CURB RAMPS ARE USED IN CONJUNCTION WITH A SHARED USE PATH, THE MINIMUM WIDTH SHALL BE THE WIDTH OF THE SHARED USE PATH
- WHEN ONLY ONE CURB RAMP IS PROVIDED FOR TWO CROSSINGS (DIAGONAL), À 4'x 4'LANDING AREA SHALL BE PROVIDED TO MANEUVER A WHEELCHAIR INTO THE CROSSWALK WITHOUT GOING INTO THE TRAVELWAY. THIS 4'x 4 LANDING AREA MAY INCLUDE THE GUTTER PAN.

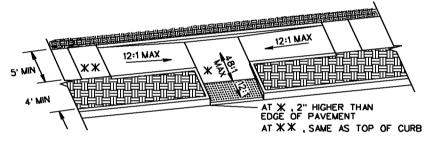




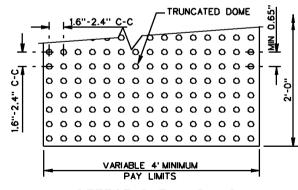
TYPE A PERPENDICULAR



PARALLEL



TYPE C PARALLEL & PERPENDICULAR



DETECTABLE WARNING DETAIL

****VDOT

ROAD AND BRIDGE STANDARDS

SHEET 1 OF 5 REVISION DATE 10/09

203.05

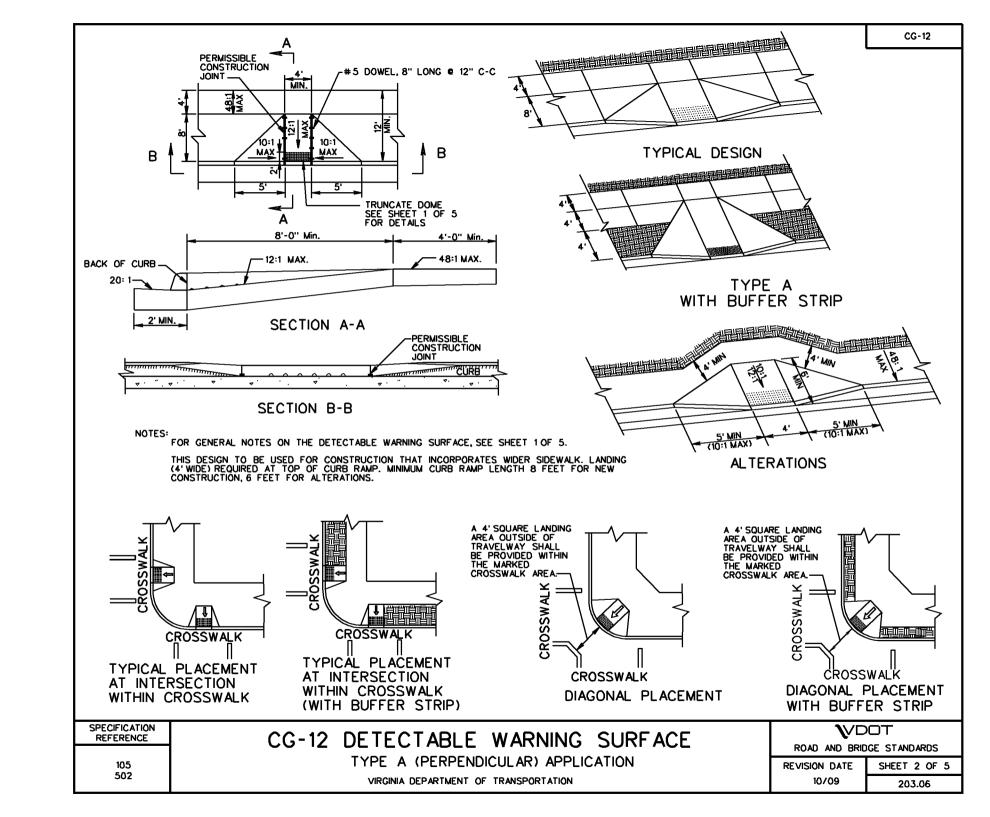
CG-12 DETECTABLE WARNING SURFACE

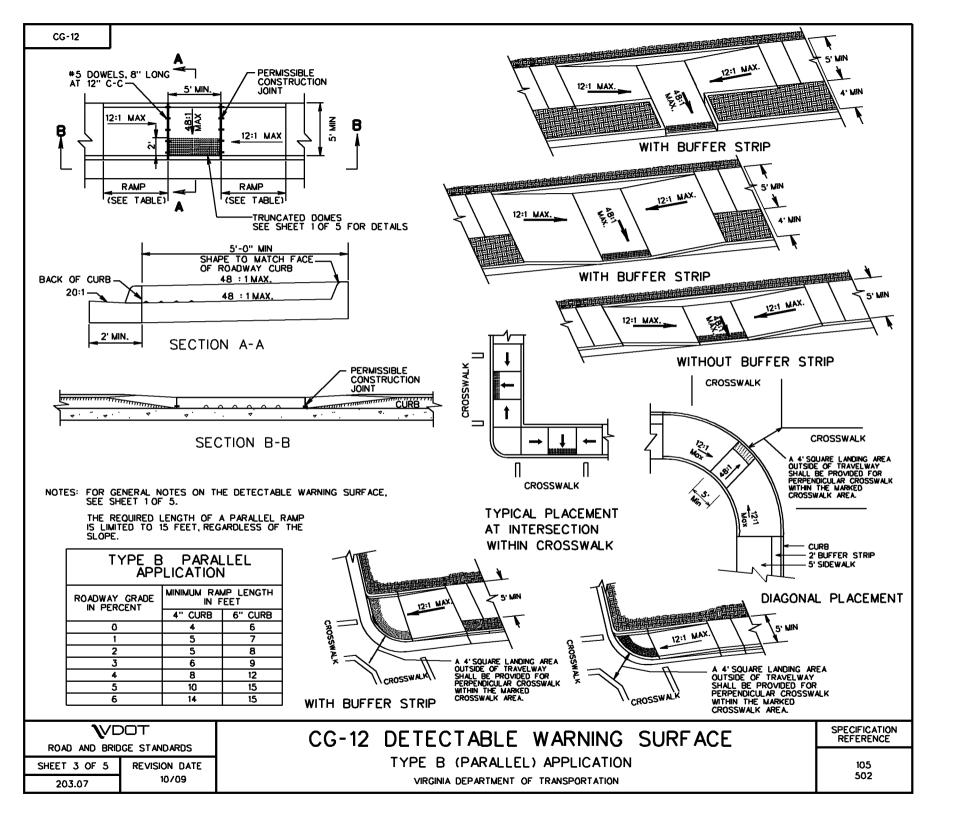
(GENERAL NOTES)

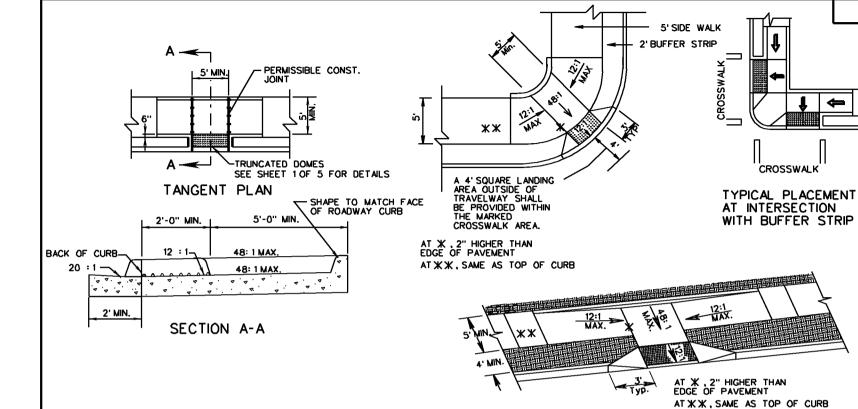
VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE

> 105 502

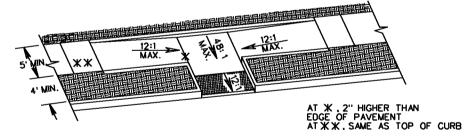






PARALLEL	TYPE C & PERPEN PLICATION	IDICULAR
ROADWAY GRADE IN PERCENT	MINIMUM RA IN F	MP LENGTH EET
IIV I CINOCIVI	4" CURB	6" CURB
0	2	4
1	2	5
2	3	5
3	3	6
4	4	8
5	5	10
6	7	14
7	13	15
8	15	15

THE REQUIRED LENGTH OF A PARALLEL RAMP IS LIMITED TO 15 FEET, REGARDLESS OF THE SLOPE.



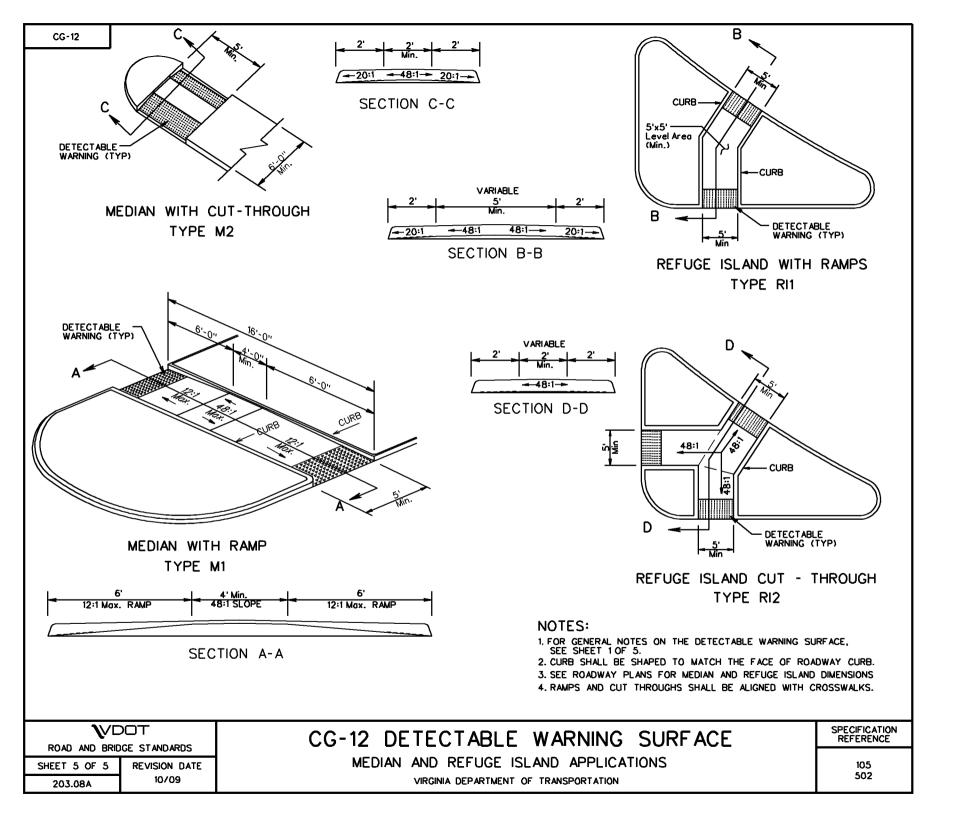
CG-12

NOTES: FOR GENERAL NOTES ON THE DETECTABLE WARNING SURFACE, SEE SHEET 1 OF 5.

> THE SELECTION OF CURB TYPE AND THE CONFIGURATION OF THE BUFFER STRIP MAY VARY TO MEET EXISTING FIELD CONDITIONS AND ROADWAY GEOMETRICS PROVIDING THE DIMENSIONS AND SLOPES ARE AS NOTED.

THIS COMBINED (PARALLEL & PERPENDICULAR) DESIGN CAN BE USED WITH ADJOINING BUFFER STRIP. LANDING AT BOTTOM OF TWO SLOPING SIDES WITH 5'X 5'MIN. DIMENSIONS. THE SHORT PERPENDICULAR RUN TO THE STREET CAN BE PROTECTED BY A LANDSCAPED SETBACK OR CONNECTED TO THE SIDEWALK WITH A WARPED SURFACE.

SPECIFICATION REFERENCE	CG-12 DETECTABLE WARNING SURFACE	ROAD AND BRID	
105	TYPE C (PARALLEL & PERPENDICULAR) APPLICATION	REVISION DATE	SHEET 4 OF 5
502	VIRGINIA DEPARTMENT OF TRANSPORTATION	10/09	203.08



PR-2 PLAIN AND REINFORCED CONCRETE PAVEMENT SHOWING REINFORCEMENT, LONGITUDINAL AND TRANSVERSE JOINTS PLAIN AND REINFORCED CONCRETE PAVEMENT SHOWING REINFORCEMENT, LONGITUDINAL AND TRANSVERSE JOINTS PLAIN AND REINFORCED CONCRETE PAVEMENT SHOWING REINFORCEMENT, LONGITUDINAL AND TRANSVERSE JOINTS STANDARD LOAD TRANSFER ASSEMBLY CONTRACTION JOINT STANDARD LOAD TRANSFER ASSEMBLY EXPANSION JOINT PR-3 8" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (WIRE MESH REINFORCEMENT) 8" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (FOR USE WITH BAR OR WIRE MESH REINFORCEMENT) 8" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (LEAVE OUT JOINT DETAIL) PR-4 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (FOR USE WITH BAR REINFORCEMENT ONLY) 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (FOR USE WITH BAR REINFORCEMENT ONLY) 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (LEAVE OUT JOINT DETAIL) PR-5 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (LEAVE OUT JOINT DETAIL) PR-6 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.01 301.02
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PR-5 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE PR-6 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.11
9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE PR-6 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.12
9" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.13
PR-6 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE 10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.14
10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.15
<u> </u>	301.16
	301.17
10" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.18
PR-7 11" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.19
11" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.20
11" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.21
PR-8 12" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.22
12" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.23
12" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.24
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13" CONTINUOUSLY REINFORCED CONCRETE PAVEMENT 14 FOOT TRAVEL LANE	301.26
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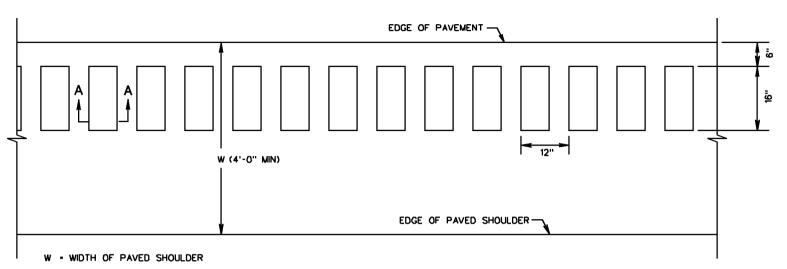
VIRGINIA DEPARTMENT OF TRANSPORTATION

VDOT ROAD AND BRIDGE STANDARDS

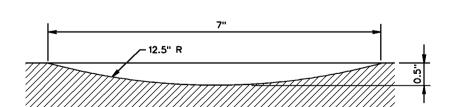
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300.01

DIRECTION OF TRAFFIC -



PLAN VIEW



SECTION A-A

NOTES

- 1. RUMBLE STRIPS SHALL BE PLACED CONTINUOUSLY AS DIRECTED BY THE ENGINEER.
- 2. RUMBLE STRIPS SHALL NOT BE PLACED WITHIN LIMITS OF BRIDGE DRAINAGE APRONS OR SPECIAL DESIGN SHOULDER SLOT INLETS.
- 3. RUMBLE STRIPS SHALL BE PLACED ON MAINLINE SHOULDERS ONLY.
- 4. FOLLOWING CUTTING AND CLEANING DEPRESSIONS OF WASTE MATERIAL, THE ENTIRE RUMBLE STRIP AREA SHALL BE COATED WITH LIQUID ASPHALT COATING (EMULSION) USING A PRESSURE DISTRIBUTOR AT AN APPROXIMATE RATE OF 0.1 GALLON PER SQUARE YARD. OVERSPRAY SHALL NOT EXTEND MORE THAN 2 INCHES BEYOND THE WIDTH OF CUT AND/OR SHALL NOT COME IN CONTACT WITH PAVEMENT MARKINGS.
- RUMBLE STRIPS SHALL NOT BE PLACED WITHIN 50 FEET OF ANY INTERSECTION, TURN LANE, ACCELERATION/DECELERATION LANE OR GORE AREA.
- 6. PAVEMENT MARKINGS SHALL BE PLACED AS DIRECTED BY THE ENGINEER.

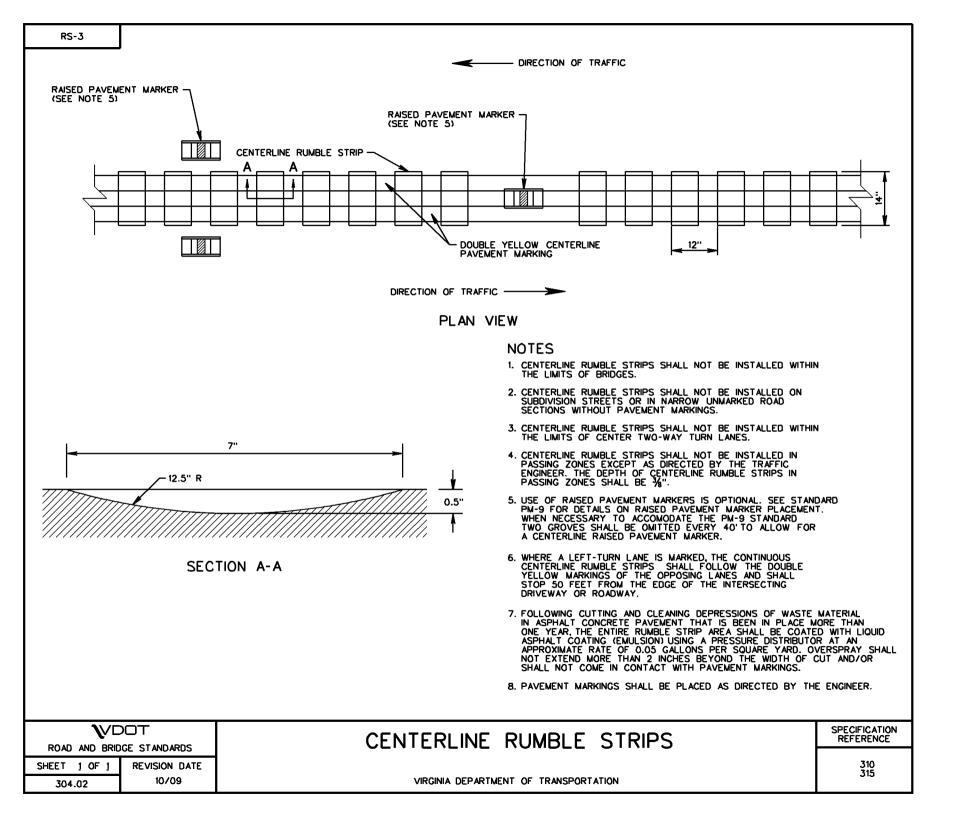
SPECIFICATION REFERENCE CONTINUOUS SHOULDER RUMBLE STRIPS

310
315

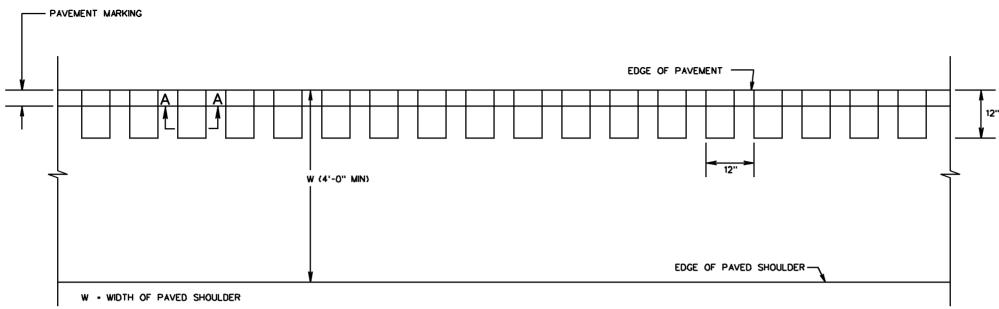
VIRGINIA DEPARTMENT OF TRANSPORTATION

ROAD AND BRIDGE STANDARDS

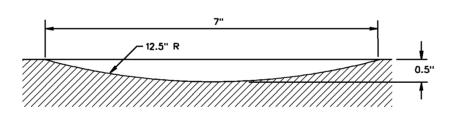
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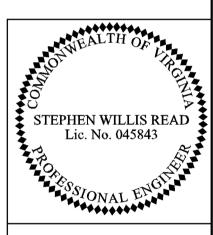
PLAN VIEW



SECTION A-A

NOTES

- RUMBLE STRIPES SHALL BE PLACED CONTINUOUSLY AS DIRECTED BY THE ENGINEER.
- RUMBLE STRIPES SHALL NOT BE PLACED WITHIN LIMITS OF BRIDGE DRAINAGE APRONS OR SPECIAL DESIGN SHOULDER SLOT INLETS.
- RUMBLE STRIPES SHALL BE PLACED ON MAINLINE SHOULDERS ONLY.
- 4. FOLLOWING CUTTING AND CLEANING DEPRESSIONS OF WASTE MATERIAL IN ASPHALT CONCRETE PAVEMENT THAT IS BEEN IN PLACE MORE THAN ONE YEAR, THE ENTIRE RUMBLE STRIP AREA SHALL BE COATED WITH LIQUID ASPHALT COATING (EMULSION) USING A PRESSURE DISTRIBUTOR AT AN APPROXIMATE RATE OF 0.05 GALLONS PER SQUARE YARD. OVERSPRAY SHALL NOT EXTEND MORE THAN 2 INCHES BEYOND THE WIDTH OF CUT AND/OR SHALL NOT COME IN CONTACT WITH PAVEMENT MARKINGS.
- 5. RUMBLE STRIPES SHALL NOT BE PLACED WITHIN 50 FEET OF ANY INTERSECTION, TURN LANE, ACCELERATION/DECELERATION LANE OR GORE AREA.
- 6. PAVEMENT MARKINGS SHALL BE PLACED AS DIRECTED BY THE ENGINEER.



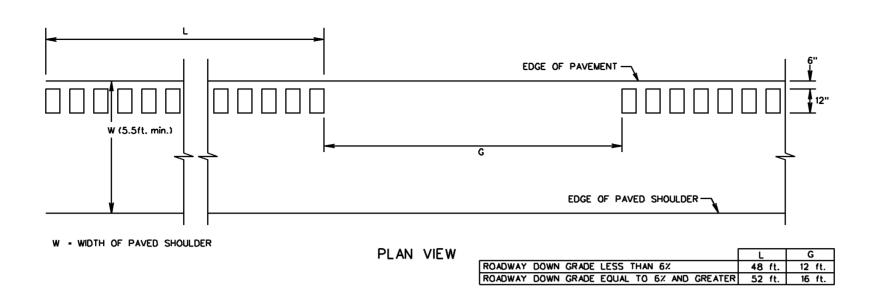
VDOT - Traffic Engineering Richmond, Virginia Traffic Engineer

VIRGINIA DEPARTMENT OF TRANSPORTATION

CONTINUOUS SHOULDER RUMBLE STRIPES

STANDARD	REVISION DATE	SPECIFICATION REFERENCE	SHEET 1 OF 1
RS-4		310 315	304.03



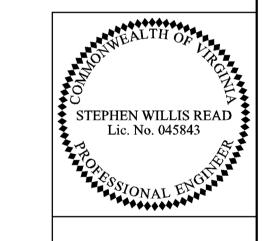


DIRECTION OF TRAFFIC

SECTION A-A

NOTES

- 1. RUMBLE STRIPS SHALL BE PLACED WITH AN INTERMITTENT PATTERN AS SHOWN IN THE PLAN VIEW AND SHALL BE INSTALED IN THE LOCATIONS SHOWN ON THE PLANS, OR AS DIRECTED BY THE FUCINEFER.
- RUMBLE STRIPS SHALL NOT BE PLACED WITHIN LIMITS OF BRIDGE DRAINAGE APRONS OR SPECIAL DESIGN SHOULDER SLOT INLETS.
- 3. RUMBLE STRIPS SHALL BE PLACED ON MAINLINE SHOULDERS ONLY.
- 4. INTERMITTENT RUMBLE STRIPS SHALL BE PLACED ON OUTSIDE SHOULDERS ONLY.
- 5. FOLLOWING CUTTING AND CLEANING DEPRESSIONS OF WASTE MATERIAL, THE ENTIRE RUMBLE STRIP AREA SHALL BE COATED WITH LIQUID ASPHALT COATING (EMULSION) USING A PRESSURE DISTRIBUTOR AT AN APPROXIMATE RATE OF 0.1 GALLON PER SQUARE YARD. OVERSPRAY SHALL NOT EXTEND MORE THAN 2 INCHES BEYOND THE WIDTH OF CUT AND/OR SHALL NOT COME IN CONTACT WITH PAVEMENT MARKINGS.
- RUMBLE STRIPS SHALL NOT BE PLACED WITHIN 50 FEET OF ANY INTERSECTION, TURN LANE, ACCELERATION/DECELERATION LANE OR GORE AREA.
- 7. PAVEMENT MARKINGS SHALL BE PLACED AS DIRECTED BY THE ENGINEER.



VDOT - Traffic Engineering Richmond, Virginia Traffic Engineer

VIRGINIA DEPARTMENT OF TRANSPORTATION

INTERMITTENT SHOULDER RUMBLE STRIPS

STANDARD	REVISION DATE	SPECIFICATION REFERENCE	SHEET 1 OF 1
RS-5		310 315	304.04

CAPACITY: HS20-44 LOADING AND ALTERNATE MILITARY LOADING.

SPECIFICATIONS:

CONSTRUCTION - VA. DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE DESIGN - AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 1983, INCLUDING INTERIM SPECIFICATIONS, 1984, 1985 AND VDOT

MODIFICATIONS, USING LOAD FACTOR DESIGN.

ALL CONCRETE SHALL BE CLASS A4.

DEFORMED REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. ALL REINFORCING BAR DIMENSIONS ON THE DETAILED DRAWINGS ARE TO CENTERS OF BARS EXCEPT WHERE OTHERWISE NOTED AND ARE SUBJECT TO FABRICATION AND CONSTRUCTION TOLERANCES.

CONSTRUCTION JOINTS SHALL BE CONSTRUCTED AND BONDED IN ACCORDANCE WITH THE CURRENT ROAD AND BRIDGE SPECIFICATIONS.

BARRELS MORE THAN 35'IN LENGTH SHALL BE POURED IN SECTIONS BY PROVIDING VERTICAL CONSTRUCTION JOINTS, NOT EXCEEDING 25 BETWEEN JOINTS NOR MORE THAN 30 FROM ENDS OF BARRELS.

**BARS BH1 & BH3 SHALL HAVE A PIN DIAMETER OF 24 BAR DIAMETERS.

DIMENSIONS ON BAR DIAGRAMS ARE OUT-TO-OUT OF BARS. BARS ARE STRAIGHT AND ${f .4}$ IN SIZE UNLESS OTHERWISE SHOWN. BL2 SHALL BE ${f .3}$ IN SIZE.

THE NUMBER OF BL1 & BL2 BARS SHOWN IN THE TABLE IS THE NUMBER OF LONGITUDINAL BARS SHOWN IN THE TYPICAL SECTION AND MAY NOT EQUAL THE TOTAL NUMBER OF BARS REQUIRED. BL1 & BL2 SHALL HAVE A LAP OF 30 BAR DIAMETERS AT SPLICES. AT CON-STRUCTION JOINTS, FIRST PLACED BARS SHALL PROJECT 30 BAR DIAMETERS BEYOND THE JOINT. ESTIMATED QUAN./LF SHOWN FOR REINFORCING STEEL DOES NOT INCLUDE QUANTITY FOR LAPS OF BL1 & BL2 BARS. THE ADDITIONAL WEIGHT PER LONGITUDINAL LAP IS SHOWN IN THE

THE CENTERS OF MAIN REINFORCING BARS SHALL BE 2" FROM THE FACE OF THE CONCRETE.

WHEN CONCRETE PROTECTIVE COATING IS REQUIRED, ALL STEEL SHALL BE EPOXY-COATED. ALL REINFORCING STEEL FOR CULVERTS UNDER 0 TO 2 FOOT FILLS SHALL BE EPOXY-COATED.

AT THE CONTRACTOR'S OPTION, BVI BARS MAY BE SPLICED AT THE PERMISSIBLE CONSTRUCTION JOINT IN ORDER TO FACILITATE CONSTRUCTION. NO ADDITIONAL COMPENSATION SHALL BE PROVIDED FOR THE INCREASE IN REINFORCING STEEL QUANTITY DUE TO THE SPLICES.

HEADWALL QUANTITIES SHOWN ASSUME WINGWALLS ARE TO BE BUILT AT A* 45 ANGLE TO THE HEADWALL.

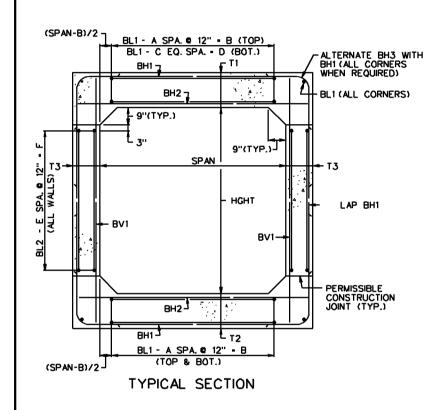
THE DESIGNS ARE APPLICABLE TO THE FILL HEIGHT AND OTHER CONDITIONS INDICATED. ANY CHANGE IN THE CONDITIONS INVALIDATES THESE DESIGNS.

WINGWALLS REFERENCED BY LETTER APPLY WHEN THE ACCEPTABLE FOUNDATION LEVEL IS THE SAME FOR BOTH BOX AND WINGS. IF FOUNDATION LEVELS ARE DIFFERENT, THE HEIGHT OF THE WINGWALL SHALL BE ADJUSTED BY SELECTION OF ANOTHER LETTERED WINGWALL OF APPRO-PRIATE HEIGHT. FOR WINGWALL DETAILS, REFER TO STANDARD SERIES BCW FOR THE APPRO-PRIATE FILL SLOPE.

FOR DETAILS OF EXTENDING EXISTING BOXES, REFER TO STANDARD BCE-01.

FOR MODIFICATION OF DETAILS FOR SKEWED CULVERTS, SEE THE SKEWED BOX DETAILS INCLUDED IN THE ROAD PLANS.

THIS STANDARD SHALL BE USED WITH THE BCB STANDARD SERIES.



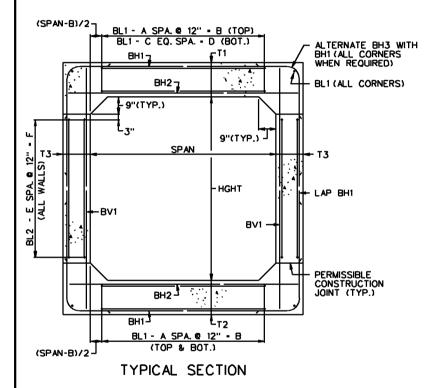
SPECIFICATION OVERSIZE BOX CULVERTS REFERENCE STANDARD DETAILS

****VDOT ROAD AND BRIDGE STANDARDS REVISION DATE SHEET 1 OF 2

10/09

1001.01

VIRGINIA DEPARTMENT OF TRANSPORTATION



CAPACITY: HS20-44 LOADING AND ALTERNATE MILITARY LOADING

SPECIFICATIONS:

CONSTRUCTION - VIRGINIA DEPARTMENT OF TRANSPORTATION ROAD AND BRIDGE SPECIFICATIONS, 2007

DESIGN - AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES 1983, INCLUDING INTERIM, SPECIFICATIONS, 1984, 1985, AND VDOT MODIFICATIONS, USING LOAD FACTOR DESIGN.

ALL CONCRETE SHALL BE CLASS A4

DEFORMED REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. ALL REINFORCING BAR DIMENSIONS ON THE DETAILED DRAWINGS ARE TO CENTERS OF BARS EXCEPT WHERE OTHERWISE NOTED AND ARE SUBJECT TO FABRICATION AND CONSTRUCTION TOLERANCES.

CONSTRUCTION JOINTS SHALL BE CONSTRUCTED AND BONDED IN ACCORDANCE WITH THE CURRENT ROAD AND BRIDGE SPECIFICATIONS.

BARRELS MORE THAN 35'IN LENGTH SHALL BE POURED IN SECTIONS BY PROVIDING VERTICAL CONSTRUCTION JOINTS, NOT EXCEEDING 25'BETWEEN JOINTS NOR MORE THAN 30' FROM ENDS OF BARRELS.

** BARS BH1 & BH3 SHALL HAVE A PIN DIAMETER OF 24 BAR DIAMETER.

DIMENSIONS ON BAR DIAGRAMS ARE OUT-TO-OUT OF BARS. BARS ARE STRAIGHT AND #4 IN SIZE UNLESS OTHERWISE SHOWN. BL2 SHALL BE #3 IN SIZE.

THE NUMBER OF BL1 & BL2 BARS SHOWN IN THE TABLE IS THE NUMBER OF LONGITUDINAL BARS SHOWN IN THE TYPICAL SECTION AND MAY NOT EQUAL THE TOTAL NUMBER OF BARS REQUIRED. BL1 & BL2 SHALL HAVE A LAP OF 30 BAR DIAMETERS AT SPLICES. AT CONSTRUCTION JOINTS, FIRST PLACED BARS SHALL PROJECT 30 BAR DIAMETERS BEYOND THE JOINT. ESTIMATED QUAL./LF SHOWN FOR REINFORCING STEEL DOES NOT INCLUDE QUANTITY FOR LAPS OF BL1 & BL2 BARS. THE ADDITIONAL WEIGHT PER LONGITUDINAL LAP IS SHOWN IN THE TABLE.

THE CENTERS OF MAIN REINFORCING BARS SHALL BE 2" FROM THE FACE OF THE CONCRETE.

WHEN CONCRETE PROTECTIVE COATING IS REQUIRED, ALL STEEL SHALL BE EPOXY-COATED. ALL REINFORCING STEEL FOR CULVERTS UNDER 0 TO 2 FOOT FILLS SHALL BE EPOXY-COATED.

AT THE CONTRACTOR'S OPTION, BV1 BARS MAY BE SPLICED AT THE PERMISSIBLE CONSTRUCTION JOINT IN ORDER TO FACILITATE CONSTRUCTION. NO ADDITIONAL COMPENSATION SHALL BE PROVIDED FOR THE INCREASE IN REINFORCING STEEL QUANTITY DUE TO THE SPICES.

HEADWALL QUANTITIES SHOWN ASSUME WINGWALLS ARE TO BE BUILT AT A 45° ANGLE TO THE HEADWALL.

THE DESIGNS ARE APPLICABLE TO THE FILL HEIGHT AND OTHER CONDITIONS INDICATED. ANY CHANGE IN THE CONDITIONS INVALIDATES THESE DESIGNS.

WINGWALLS REFERENCED BY LETTER APPLY WHEN THE ACCEPTABLE FOUNDATION LEVEL IS THE SAME FOR BOTH BOX AND WINGS. IF FOUNDATION LEVELS ARE DIFFERENT, THE HEIGHT OF THE WINGWALL SHALL BE ADJUSTED BY SELECTION OF ANOTHER LETTERED WINGWALL OR APPROPRIATE HEIGHT. FOR WINGWALL DETAILS, REFER TO STANDARD SERIES BCW FOR THE APPROPRIATE FILL SLOPE.

FOR DETAILS OF EXTENDING EXISTING BOXES, REFER TO STANDARD BCE-01.

FOR MODIFICATIONS OF DETAILS FOR SKEWED CULVERTS, SEE THE SKEWED BOX DETAILS INCLUDED IN THE ROAD PLANS.

THIS STANDARD SHALL BE USED WITH THE BCS STANDARD SERIES.

ROAD AND BRIDGE STANDARDS

SHEET 1 0F 2 REVISION DATE

1002.01 10/09

SINGLE BOX CULVERTS

STANDARD DETAILS

VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE

Capacity: HS20-44 Loading and Alternate Military Loading. Specifications:

Construction - Va. Department of Transportation Road and Bridge Specifications 2007.

Design - AASHTO Standard Specifications for Highway Bridges, 1983, including Interim Specifications, 1984, 1985 and VDOT Modifications, using Load Factor Design.

All concrete shall be Class A4.

Deformed reinforcing bars shall conform to ASTM A615, Grade 60.

All reinforcing bar dimensions on the detailed drawings are to centers of bars except where atherwise noted and are subject to fabrication and construction tolerances.

Construction joints shall be constructed and bonded in accordance with the current Road and Bridge Specifications.

Barrels more than 35' in length shall be poured in sections by providing vertical construction joints, not exceeding 25' between joints nor more than 30' from ends of barrels.

** All bends shall be made with a pin diameter as listed in TABLE A except for Bars BH3 & BH4 which shall have a pin diameter of 24 bor diameters.

Dimensions on bar diagrams are out-to-out of bars. Bars are straight and *4 in size unless otherwise shown. BL2 shall be *3 in size.

The number of BL1 & BL2 bars shown in the table is the number of longitudinal bars shown in the Typical Section and may not equal the total number of bars required. BL1 & BL2 shall have a lop of 30 bar diameters at splices. At construction joints, first placed bars shall project 30 bar diameters beyond the joint. Estimated QUAN./LF shown for reinforcing steel does not include quantity for laps of BL1 & BL2 bars. The additional weight per longitudinal lap is shown in the table.

The centers of main reinforcing bars shall be 2" from the face of the concrete.

When concrete protective coating is required, all steel shall be epoxy-coated. All reinforcing steel for culverts under 0 to 2 foot fills shall be epoxy-coated.

At the Contractor's option, BV1 & BV2 bors may be spliced at the permissible construction joint in order to facilitate construction. No additional compensation shall be provided for the increase in reinforcing steel quantity due to the splices.

Bar HW2 shall be 4" less than culvert height in length.

Headwall quantities shown assume wingwalls are to be built at a 45° angle to the headwall.

The designs are applicable to the fill height and other conditions indicated. Any change in the conditions invalidates these designs.

Wingwalls referenced by letter apply when the acceptable foundation level is the same for both box and wings. If foundation levels are different, the height of the wingwall shall be adjusted by selection of another lettered wingwall of appropriate height. For wingwall details, refer to standard series BCW for the appropriate fill slope.

For details of extending existing boxes, refer to Standard BCE-01.

For modification of details for skewed culverts, see the Skewed Box

Details included in the road plans.

This standard shall be used with the BCD standard series.

SPECIFICATION REFERENCE

DOUBLE BOX CULVERTS

STANDARD DETAILS

VIRGINIA DEPARTMENT OF TRANSPORTATION

****VDOT

ROAD AND BRIDGE STANDARDS

REVISION DATE 10/09

Capacity: HS20-44 Loading and Alternate Military Loading.

Specifications:

Construction - Vo. Department of Transportation Road and Bridge Specifications, 2007.

Design - AASHTO Standard Specifications for Highway Bridges, 1983, including Interim Specifications, 1984, 1985 and VDOT Modifications, using Load Factor Design.

All concrete shall be Class A4.

Deformed reinforcing bars shall conform to ASTM A615, Grade 60. All reinforcing bar dimensions on the detailed drawings are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

Construction joints shall be constructed and bonded in accordance with the current Road and Bridge Specifications.

Barrels more than 35' in length shall be poured in sections by providing vertical construction joints, not exceeding 25' between joints nor more than 30' from ends of barrels.

** All bends shall be made with a pin diameter as listed in TABLE A except for Bars BH3 & BH4 which shall have a pin diameter of 24 bor diameters.

Dimensions on bar diagrams are out-to-out of bars. Bars are straight and *4 in size unless otherwise shown. BL2 shall be *3 in size.

The number of BL1 & BL2 bars shown in the table is the number of longitudinal bars shown in the Typical Section and may not equal the total number of bars required. BL1 & BL2 shall have a lap of 30 bar diameters at splices. At construction joints, first placed bars shall project 30 bar diameters beyond the joint. Estimated QUAN./LF shown for reinforcing steel does not include quantity for laps of BL1 & BL2 bars. The additional weight per longitudinal lap is shown in the table.

The centers of main reinforcing bars shall be 2" from the face of the concrete.

When concrete protective coating is required, all steel shall be epoxy-coated. All reinforcing steel for culverts under 0 to 2 foot fills shall be epoxy-coated.

At the Contractor's option, BV1 & BV2 bars may be spliced at the permissible construction joint in order to facilitate construction. No additional compensation shall be provided for the increase in reinforcing steel quantity due to the splices.

Bar HW2 shall be 4" less than culvert height in length.

Headwall quantities shown assume wingwalls are to be built at a 45° angle to the headwall.

The designs are applicable to the fill height and other conditions indicated. Any change in the conditions invalidates these designs.

Wingwalls referenced by letter apply when the acceptable foundation level is the same for both box and wings. If foundation levels are different, the height of the wingwall shall be adjusted by selection of another lettered wingwall of appropriate height. For wingwall details, refer to standard series BCW for the appropriate fill slope.

For details of extending existing boxes, refer to Standard BCE-01.

For modification of details for skewed culverts, see the Skewed Box Details included in the road plans.

This standard shall be used with the BCT standard series.

TRIPLE BOX CULVERTS

STANDARD DETAILS

VIRGINIA DEPARTMENT OF TRANSPORTATION

****VDOT

ROAD AND BRIDGE STANDARDS

REVISION DATE 10/09 SHEET 2 OF 3

1004.02

	DIME	NSION	S														RE	NF()	RCING	STEEL									
SPAN HGH	T TI	T2	Т3	T4								Bł	11		Вн	2			ВНЗ				ВН	4		B\	/1		BV2
(FT.)	TOP SLAB	BOTTOM SLAB	EXTERIOR WALL (IN.)	INTERIOR WALL (IN.)	A	В	С	В	E	F	SIZE	SPACING c-c	LENGTH	SIZE	SPACING c-c	LENGTH	SIZE	SPACING c-c	a	LENGTH	SIZE	SPACING C-C	a	Ь	LENGTH	SIZE SPACING c-c	LENGTH	SIZE	o-c C-C LENGTH
3 3	81/4	81/4"	8"	8.	9	9'- 0"	9	9'- 0"	Т	I'- 0"	4	12"	11'- 2"	4	12"	11'- 4"	4	12"	2'- 11/4"	3'-11"	4	12"	2'- 11/4"	2'- 8'/4"	4'- 6"	4 12"	4'- 0"	4 1	2" 4'-
3 4	81/4	8 ¹ /4"	8"	8.	9	3'- 0"	9	9'- 0"	2	2'- 0"	4	12"	11'- 2"	4	12"	11'- 4"	4	12"	2'- 11/4	3'-11"	4	12"	2'- 11/4"	3 2/4	5'- 0"	4 12"	5'- 0"	4 1	2" 5'-
4 3	101/2	101/2"	8-	8"	12	12'- 0"	12	12'- 0"	-	1'- 0"	4	12"	13'- 9"	4	12"	14"- 4"					4	12"	2'- 3%	2'-101/2"	4'-11"	4 12"	4'- 5"	4 1	2" 4'-
4 4	101/2	101/2"	8-	8.	12	12'- 0"	12	12'- 0"	2	2'- 0"	4	12"	13'- 9"	4	12"	14"- 4"					4	12"	2'- 3%	3 - 41/2"	5'- 5"	4 12"	5'- 5"	4 1	2" 5'-
4 5	10¾	10¾*	8"	8.	12	12'- 0"	12	12'- 0"	3	3'- 0"	4	12"	13'- 9"	4	12"	14'- 4"					4	12"	2'- 3%	3'-10%	5'-11"	4 12"	6'- 5"	4 1	2" 6'-
4 6	10¾	10¾*	8.	8.	12	12'- 0"	12	12'- 0"	4	4'- 0"	4	12"	13'- 9"	4	12"	14'- 4"	4	12"	2 - 3%	4'- 4"	4	12"	2'- 3%"	4 - 4%	6'- 5"	4 11*	7'- 5"	4 1	2" 7'-
5 3	121/2	121/2"	8-	8.	15	15'- 0"	15	15'- 0"	- 1	1'- 0"	4	10"	16'- 4"	4	10	17'- 4"					4	10"	2'- 6"	3'- 1/2"	5'- 3"	4 12"	4'- 9"	4 1	2" 4'-
5 4	/2	121/2"	8-	8.	15	15'- 0"	15	15'- 0"	2	2'- 0"	4	10"	16'- 4"	4	9	17'- 4"					4	10"	2'- 6"	3'- 61/2"	5'- 9"	4 12"	5'- 9"	4 1	2" 5'-
5 5	121/2	121/2"	8"	8"	15	15'- 0"	15	15'- 0"	3	3'- 0"	4	10"	16'- 4"	4	5	17'- 4"				-	4	10"	2'- 6"	4'- 1/2"	6'- 3"	4 12"	6'- 9"	4 1	2" 6'-
5 6	12/2	121/2"	8"	8"	15	15'- 0"	15	15'- 0"	4	4'- 0"	4	10"	16'- 4"	4	10"	17'- 4"					4	10"	2'- 6"	4'- 6'/2"	6'- 9"	4 12"	7'- 9"	4 I	2" 7'-
5 7	121/2	121/2"	8-	8.	15	15'- 0"	15	15'- 0"	5	5'- 0"	4	10"	16'- 4"	4	10-	17'- 4"					5	10"	2'- 6"	5'- 1/2"	7'- 3"	4 71/2"	8'- 9"	4 1	2" 8'-
6 4	- 79	14¾*	8-	8.	18	18'- 0"	18	18'- 0"	2	2'- 0"	4	3.	18'-11"	5	9"	20'- 4"					4	3.	2'- 8%	3'- 8¾"	6'- 2"	4 12"	6'- I"	4 1	2" 6'-
6 5			8-	8.	18	18 0.	18	18'- 0"	3	3 0.	4	9"	1811.	5	9"	20'- 4"					4	9"	2'- 8¾"	4'- 2¾	6'- 8"	4 12"	7'- 1"	—	2" 7'-
6 6		14¾"	8"	8"	18	18'- 0"	18	18'- 0"	4	4'- 0"	4	9"	18'-11"	5	9"	20'- 4"					4	9"	2'- 8¾"	4'- 8¾"	7'- 2"	4 12"	8'- I"		2" 8'-
6 7	,,	14¾"	8"	8"	18	18'- 0"	18	18'- 0"	5	5 0	4	9"	18'-11"	5	9"	20'- 4"	3	9"	2 8%	5'- 2"	4	9"	2'- 8%		7'- 8"	4 71/2	3:- I.		2" 9'-
6 8	14¾	14¾*	8-	8"	18	18'- 0"	18	18'- 0"	6	6 0.	4	9"	18'-11"	5	9"	20'- 4"					5	9"	2'- 8¾"	5'- 8¾	8'- I"	4 51/2"	10 1.		2" 10'-
7 4	171/4"	171/4"	8-	8-	21	21'- 0"	21	21'- 0"	2	2'- 0"	4	9"	21'- 6"	5	9"	23'- 4"					4	9"	2'-10%	3'-111/4"	6'- 7"	4 12"	6'- 6"	-	01/2" 6'-
7 6	- "	171/4"	8-	8"	21	21'- 0"	21	21'- 0"	4	4'- 0"	5	9"	21'- 6"	5	9"	23'- 4"					4	9"	2'-10%	4'-111/4"	7'- 7"	4 11/2	8'- 6"		I. 8
7 8		171/4"	8"	8"	21	21'- 0"	21	21'- 0"	6	6'- 0"	5	9"	21'- 6"	5	9"	23'- 4"	4	9"	2'-10%	5'- 6"	4	9"	2'-10%'	5'-111/4"	8'- 7"	4 51/2	10'- 6"	4 1	
7 10		171/4"	9"	8"	21	21'- 0"	21	21'- 0"	8	8'- C'	5	9"	21'- 6"	5	9"	23'- 6"	3	9"	2'-11¾"	5'- 9"	5	9"	2'-11-	6'-11'//"	9'- 7"	4 4	12'- 6"		1" 12'-
8 4	19/4	19/4	8-	8.	24	24'- 0"	24	24'- 0"	2	2'- 0"	5	8"	24'- 2'	5	8-	26'- 4"					4	8-	3'- 11/4"	4 1/4	6'-11"	4 12"	6'-10"	-	0" 6'-
8 6	-77	18/4.	8-	8.	24	24'- 0"	24	24'- 0"	4	4'- 0"	4	8"	24 - 2	5	8-	26'- 4"					4	8"	3 1/4.	5'- 1'/4"	7'-11"	4 11*	8'-10"		0. 8
8 8	+ -	19"	8"	8"	24	24'- 0"	24	24'- 0"	6	6'- 0"	4	8"	24'- 2"	5	8"	26'- 4"	3	8"	3- 11/4	6'- 0"	4	8.	3'- 11/4"	6'- 1"	8'-11"	4 51/2"	1010.		0. 101
8 10		19//	9"	8.	24	24'- 0"	24	24'- 0"	8	8'- 0"	5	8"	21 2	5	8"	26'- 6"	3	8-	3'- 2'/4"	6 2	5	8"	3'- 2/4"	7 1/4	10'- 0"	4 4	12'-10"		0" 12'-1
9 4	211/4	51/4.	8-	8.	27	27'- 0"	27	27'- 0"	2	2'- 0"	6	9"	26'- 9"	6	9"	29'- 4"	<u></u>				4	9"	3'- 3%	4'- 3/4	7'- 4"	4 12	7'- 3"	-	0" 7'-
9 6	+ - / -	211/4"	8"	8"	27	27'- 0"	27	27'- 0"	4	4'- 0"	6	9"	26'- 9"	6	9"	29'- 4"					4	9"	3'- 3%	5 3/4	8'- 4"	4 11*	9'- 3"		0" 9'-
9 8	+ - /,	211/4"	8"	8.	27	27'- 0"	27	27'- 0"	6	6'- 0"	6	9"	26'- 9"	6	9"	29'- 4"					5	9"	3'- 3%"	6 3/4	9'- 3"	6 11/2	11 3.		0"
9 10	- "	211/4"	9"	8.	27	27'- 0"	27	27'- 0"	8	8 0.	6	9"	26'- 9"	6	9"	29'- 6"					6	9"	3'- 4%	7 - 3/4	10'- 3"	5 6"	13'- 3"		0" 13'-
9 12		211/2"	101/2"	8"	27	27'- 0"	27	27'- 0"	10	10'- 0"	6	9"	26'- 9"	6	9"	29'- 9"					1	9"	3'- 6/8"	8 - 3/2	- 4	7 10"	15'- 3"		0" 15'-
10 4	1 20/4	23'/4"	8"	8.	30	30'- 0"	30	30'- 0"	2	2'- 0"	6	9"	29'- 4"	6	9"	32'- 4"					4	9"	3'- 6"	4'- 5/4	7'- 8"	4 12"	7'- 6"	-	0" 7'-
10 8	23/4	231/4"	8"	8"	30	30'- 0"	30	30'- 0"	4	4'- 0"	6	9"	29'- 4"	6	9"	32'- 4"	<u> </u>				4	9"	3'- 6"	5'- 5/4"	8'- 8"	4 101/2"	9-6	-	0" 9'-
10 8		231/4"	8"	8"	30	30'- 0"	30	30'- 0"	6	6'- 0"	6	9"	29'- 4"	6	9"	32'- 4"	3	9"	3'- 6"	6'-10"	4	9"	3'- 6"	6'- 51/4"	9'- 8"	6 111/2"	11'- 6'	-	0" 11'-
10 10	1	23'/4"	9"	8"	30	30'- 0"	30	30'- 0"	8	8'- 0"	6	9"	29'- 4"	6	9"	32'- 6'	3	9"	3'- 7"	7'- 0"	5	9"	3'- 7"	7'- 5/4"	10'- 8"	5 6"	13'- 6"		0" 13'-
10 12		231/2	101/2"	8"	30	30'- 0"	30	30'- 0"	10	10'- 0"	6	9"	29'- 4"	6	9"	32'- 9"	3	9"	3'- 81/2"	7'- 3"	6	9"	3'- 81/2"	8'- 5/2	11'- 9"	5 5"	15'- 7"		0" 15'-
12 6	- '-	271/2"	8"	8"	36	36'- 0"	36	36'- 0"	4	4'- 0"	7	8"	34'- 6"	6	8"	38'- 4"	<u> </u>				4	8"	3'-10%'	5 - 9/2	9'- 5"	4 9"	10 3.	-	81/2" 10'-
12 8	/2	271/2"	8"	8"	36	36'- 0"	36	36'- 0"	6	6'- 0"		8"	34'- 6"	6	8"	38'- 4"	3	8-	3'-10%	7'- 7"	1	8"	3'-10%	6'- 91/2	10'- 5"	5 71/2	12'- 3"	-	81/2" 12'-
12 10	- 76	_	91/4"	8"	36	36'- 0"	36	36'- 0"	8	8'- 0"	6	8"	34'- 6"	6	8"	38'- 6"	3	8"	4'- 0"	7'-10"	5	8.	4'- 0"	7 - 9/2	11'- 6"	5 6	14'- 3"		81/2" 14'-
12 12	27/2	271/2"	101/2"	8.	36	36'- 0"	36	36'- 0"	10	10 0.	6	8"	34'- 6"	6	8-	38'- 9"	3	8-	4'- 11/4"	8'- 0"	6	8"	4'- 11/4"	8 9/2	12'- 6"	6 7*	16'- 3"	4	8. 16

FOR TYPICAL SECTION, NOTES AND OTHER DETAILS, REFER TO STANDARD BCT-DT.

ROAD AND BRIDGE STANDARDS

SHEET 1 OF 2 REVISION DATE

1004.22 10/09

TRIPLE BOX CULVERTS

40 TO 45 FT. FILLS

VIRGINIA DEPARTMENT OF TRANSPORTATION

SPECIFICATION REFERENCE

ıl	REINFORCING STEEL														QUA	N./LF				HE	ADW	ALLS			$\overline{}$
SPANI	HGHT						ВТІ							NG G. JTJ		2	G		HW I			رې	3S	۶	SS.
(FT.)	(FT.)	SIZE	SPACING C-C	a	b	c	d	е	f	g	LENGTH	NO. BLI BARS	NO. BL2 BARS	REINFORCING STEEL (LBS/LONG.	CONCRETE CLASS A4 (CY/LF)	REINFORCING STEEL (LBS/LF)	HEADWALL LENGTH	SIZE	LENGTH	NO. HW2 BARS	NO. HW3 BARS	INLET CONCRETE CL. A4 (CY)	INLET REINFORCING STEEL (LBS)	OUTLET CONCRETE CL. A4 (CY)	OUTLET REINFORCING STEEL (LBS) WINGWALL
3	3	4	12"	2'- 8¾	0'- 4¾'	1'- 6¾"	0'- 6'/4"	0'- 4"	11'- 3%	1'- 5%	11'-11"	44	16	42.388	1.006	119.910	13'- 2"	6	12'-10"	2	8	1.823	102.932	1.804	77.103 A
3	4	4	12"	2'- 8¾*	0'- 4¾*	1'- 6¾'	0'- 6'/4"	0'- 4"	11'- 3%	1'- 5%	11'-11"	44	24	45.212	1.105	128.262	13'- 2"	6	12'-10"	2	10	1.928	109.835	1.896	77.103 C
4	3	4	12"	3'- 5 ¹ /4"	0'- 7"	1'- 9"	0'- 9%	0'- 6'/4	14'- 4"	1'-101/2"	15'- 2"	56	16	52.408	1.362	132.045	16'- 2"	6	15'-10"	2	8	2.085	120.956	2.076	95.127 B
4	4	4	12"	3'- 51/4"	0'- 7"	1 3.	0'- 9%	0'- 6'/4"	14'- 4"	110//5.	15'- 2"	56	24	55.232	1.460	140.397	16'- 2"	6	15'-10"	2	10	2.190	127.859	2.168	95.127 D
4	5	4	12"	3'- 51/a"	0'- 71/4"	I'- 8¾'	0'- 9¾*	0'- 61/2	14'- 4"	1,-10/4.	15'- 3"	56	32	58.056	1.581	148.861	16'- 2"	6	15'-10 "	2	12	2,289	134.761	2.255	95.127 F
4	6	4	12"	3'- 51/8"	0'- 71/4"	I'- 8¾'	0'- 9¾•	0'- 6'/2'	14'- 4"	1,-10/4.	15'- 3"	56	40	60.880	1.678	169.692	16'- 2"	6	15'-10"	2	14	2.394	141.664	2.347	95.127 H
5	3	4	10"	4'- 1%	0 3.	1'-11¾*	<u>'- ¼</u>	0'- 8'/4	17'- 4"	2'- 3¾'	18'- 6 "	68	16	62.428	1,772	170.945	19'- 2"	6	18'-10"	2	8	2.295	138.980	2.296	113.151 B
5	4	4	10"	4'- 1%	0 3.	1'-11¾*	1- 1/4	0'- 8'/4	17'- 4"	2'- 3¾'	18'- 6"	68	24	65.252	1.870	179.564	19'- 2"	6	1810.	2	10	2.401	145.883	2.389	113.151 D
5	5	4	10"	4'- 1%	0'- 9"	1'-11¾*	1'- ¼"	0'- 8'/4	17'- 4"	2 - 3%	18'- 6"	68	32	68.076	1.968	188.184	19'- 2"	6	18'-10"	2	12	2.506	152.785	2.481	113.151 F
5	6	4	10"	4'- 1%	0'- 9"	1'-11%	• • •	0'- 8'/4"	17'- 4"	2 - 31/4	18'- 6"	68	40	70.900	2.066	196.803	19'- 2"	6	18'-10"	2	14	2.611	159.688	2.574	113,151 H
5	7	4	10-	4'- 1%	0 3.	1'-11¾*	1:- 1/4"	0'- 8'/4	17'- 4"	2 - 3%	18'- 6"	68	48	73,724	2,162	225.486	19'- 2"	6	18'-10"	2	16	2,716	166.591	2.667	113,151 J
6	4	4	9"	4'-10%"	011/4.	2'- 21/4"	- 70	010//5.	20'- 4"	2'- 8¾'	21'- 9"	80	24	75.272	2.384	237.810	22'- 2"	6	21'-10"	2	10	2.563	163.907	2.561	131.175 D
6	5	4	9-	4'-10%"	011/4.	2'- 2 ¹ /4"		0101/5.	20'- 4"	2 - 8%	21'- 9"	80	32	78.096	2.482	246.608	22'- 2"	6	21'-10"	2	12	2.668	170.809	2.654	131.175 F
6	6	4	9"	4'-10%"	0'-111/4"	2'- 21/4"	,0	0'-101/2"	20'- 4"	2 - 8%	21'- 9"	80	40	80.920	2.580	255.405	22'- 2"	6	21'-10"	2	14	2.773	177.712	2.746	131.175 H
6	7	4	9"	4'-10%"	0'-111/4"	2'- 21/4"		0'-101/2"	20'- 4"	2'- 8¾'	21'- 9"	80	48	83.744	2.677	281.845	22'- 2"	6	21'-10"	2	16	2.878	184.615	2.839	131.175 J
6	8	4	9"	4'-10%	011/4.	2'- 21/4"		010//5.	20'- 4"	2 - 8¾	21'- 9"	80	56	86.568	2.774	304.790	22'- 2"	6	21'-10"	2	18	2.983	191.517	2.932	131.175 L
7	4	5	9-	5'- 6¾	1'- 1%	2'- 4%	1'- 7"	I'- I"	23'- 41/8"	3'- 1%"	25'- 1"	92	24	85.292	3.018	295.429	25'- 2"	6	24'-10"	2	10	2.689	181.931	2.697	149.199 E
7	6	4	9"	5'- 6¾		2'- 4%'	I'- 6%°	1 1.	23'- 41/8"	3'- I¾"	25'- 1"	92	40	90.940	3.214	309.518	25'- 2"	6	24'-10"	2	14	2.899	195.736	2.882	149.199 1
7	8	4	9"	5'- 6¾	I'- I ¾ '	2'- 4%'	I'- 6%"	1 1.	23'- 41/8"	3'- 1%	25'- 1"	92	56	96.588	3.408	363.278	25'- 2"	6	24'-10"	2	18	3.109	209.541	3.068	149.199 M
7	10	4	9"	5'- 7¾*	1'- 1%	2'- 4%	1'- 6%"	1 1.	23'- 6'/8"	3'- 13/8"	25'- 3"	92	72	102.236	3.681	410.043	25'- 2"	6	24'-10"	2	22	3.241	223.347	3.174	149.199 0
8	4	5	8"	6'- 3'/4"	1 - 3%	2'- 7%	1'- 9%	1'- 3"	26 - 3%	3'- 6%	28'- 5"	104	24	95.312	3.660	384.188	28'- 2"	6	27'-10"	2	10	2.781	199.955	2.799	167.223 E
8	6	5	8"	6 3 /4	1 - 3%	2'- 7%	1 - 9%	1'- 3"	26 - 3%	3'- 6%	28'- 5"	104	40	100.960	3.858	377.182	28'- 2"	6	27'-10"	2	14	2.991	213.760	2.985	167.223
8	8	5	8"	6'- 3%	1'- 35%"	2'- 7%"	1'- 9'/2"	1'- 2¾	26'- 3%"	3'- 6%	28'- 4"	104	56	106.608	4.011	425.598	28'- 2"	6	27'-10"	2	18	3.217	227.565	3.186	167.223 M
8	10	5	8"	6 - 4 /4	1'- 3%	2 - 7%	1'- 9%	1'- 3"	26'- 5%'	3'- 6%	28'- 7"	104	72	112.256	4.324	513.594	28'- 2"	6	27'-10"	2	22	3.331	241.371	3.274	167.223 Q
9	4	5	9"	6'-11%	1 - 5%	2'-10'/4"	2'- 5%"	1'- 5"	29'- 4"	3'-11¾"	31'- 8"	116	24	105.332	4.377	458.279	31'- 2"	6	30'-10"	2	10	2.855	217.979	2.883	185.247 F
9	6	5	9"	6'-11%	1'- 5%	2'-10'/4"	2'- 5%"	1'- 5"	29'- 4"	3'-11¾"	31'- 8"	116	40	110.980	4.573	478.066	31'- 2"	6	30'-10"	2	14	3.065	231.784	3.068	185.247 J
9	8	5	9"	6'-11%	1'- 5%	2'-10'/4"	2'- 3/8"	1'- 5"	29'- 4"	3'-11¾"	31'- 8"	116	56	116.628	4.766	534.043	31'- 2"	6	30'-10"	2	18	3.275	245.589	3,254	185.247 N
9	10	5	9"	7'- 1/8"	1'- 5%	2'-10'/4"	2'- %'	1'- 5"	29'- 6"	3'-11¾"	31'-10"	116	72	122.276	5.042	598.272	31'- 2"	6	30'-10"	2	22	3.403	259.395	3.356	185.247 R
9	12	5	9"	7'- 2'/4"	1'- 6'/8"	2'-10"	2'- 1"	1'- 5'/4"	29'- 9"	3'-11/2"	32'- "	116	88	127.924	5.437	673.368	31'- 2"	6	30'-10"	2	26	3.439	273.200	3.366	185.247 V
10	4	6	9"	7'- 81/2"	1'- 8"	3'- 1"	2'- 3%	I'- 7°	32'- 4"	4'- 5"	34'-11"	128	24	115.352	5,167	542,758	34'- 2"	6	33'-10"	2	10	2.873	236.003	2,911	203.271 F
10	8	6	9"	7'- 81/2"	1'- 8"	3'- 1"	2'- 3%"	1'- 7"	32'- 4"	4'- 5"	34'-11"	128	40	121.000	5.363	563.235	34'- 2"	6	33'-10"	2	14	3.083	249.808	3.097	203.271 J
10	8	6	9*	7'- 81/2"	1 8.	3'- 1"	2'- 3%	1'- 7"	32'- 4"	4'- 5"	34'-11"	128	56	126.648	5.557	614.472	34'- 2"	6	33'-10"	2	18	3.294	263.613	3.282	203.271 N
10	10	6	9"	7'- 91/2"	1'- 8"	3'- "	2'- 3%	1'- 7"	32'- 6"	4'- 5"	35'- 1"	128	72	132.296	5.836	673.740	34'- 2"	6	33'-10"	2	22	3.419	277.419	3.382	203.271 R
10	12	6	9"	7'-10%	1'- 8'/4"	3'- ¾	2'- 4"	1'- 7'/4"	32'- 9"	4'- 4¾	35'- 5"	128	88	137.944	6.238	745.759	34'- 2"	6	33'-10"	2	26	3.450	291.224	3.387	203.271 V
12	6	6	8"	9'- 1%	2'- '/4"	3'- 6%'	2'- 9%"	1-11//	38'- 41/8"	5'- 3'/8"	41'- 6"	152	40	141.040	7.222	782.525	40'- 2"	6	39'-10"	2	14	3.003	285.856	3.036	239.319 K
12	8	6	8"	9'- 1%	2'- 1/4"	3'- 6%"	2'- 9%	1-11/4	38'- 41/8"	5'- 31/8"	41'- 6"	152	56	146.688	7,415	839.828	40'- 2"	6	39'-10"	2	18	3.213	299.661	3,221	239.319 0
12	10	6		9'- 2%	2'- 1/4"	3'- 6¾"	2'- 9%	1-11/4	38'- 6%"	5'- 31/8"	41'- 9"	152	72	152.336	7.725	848.516	40'- 2"	6	39'-10"	2	22	3.312	313.467	3.294	239.319 S
12	12	6	8-	9'- 41/8"	2'- 1/4"	3'- 6 % "	2'- 9%	111/4	38'- 9 ¹ /8"	5'- 31/8"	41'-11"	152	88	157.984	8.059	933.076	40'- 2"	6	39'-10"	2	26	3.379	327.272	3.336	239.319 W

REFERENCE	TRIPLE BOX CULVERTS
	40 TO 45 FT. FILLS

40 TO 45 FT. FILLS VIRGINIA DEPARTMENT OF TRANSPORTATION

VOOT
ROAD AND BRIDGE STANDARDS

REVISION DATE 10/09 SHEET 2 OF 2 1004.23

Capacity: HS20-44 Loading and Alternate Military Loading.

Specifications:

Construction - Va. Department of Transportation Road and Bridge Specifications, 2007. Design - AASHTO Standard Specifications for Highway Bridges, 1983, including Interim Specifications, 1984, 1985 and VDOT Modifications, using Load Factor Design.

All concrete shall be Class A4.

Deformed reinforcing bars shall conform to ASTM A615, Grade 60. All reinforcing bar dimensions on the detailed drawings are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

Construction joints shall be constructed and bonded in accordance with the current Road and Bridge Specifications.

Barrels more than 35' in length shall be poured in sections by providing vertical construction joints, not exceeding 25' between joints nor more than 30' from ends of barrels.

** All bends shall be made with a pin diameter as listed in TABLE A except for BH3 & BH4 which shall have a pin diameter of 24 bar diameters.

Dimensions on bar diagrams are out-to-out of bars. Bars are straight and °4 in size unless otherwise shown. BL2 shall be °3 in size.

The number of BL1 & BL2 bars shown in the table is the number of longitudinal bars shown in the Typical Section and may not equal the total number of bars required. BL1 & BL2 shall have a lop of 30 bar diameters at splices. At construction joints, first placed bars shall project 30 bar diameters beyond the joint. Estimated QUAN./LF shown for reinforcing steel does not include quantity for laps of BL1 & BL2 bars. The additional weight per longitudinal lap is shown in the table.

The centers of main reinforcing bars shall be 2" from the face of the concrete.

When concrete protective coating is required, all steel shall be epoxy-coated. All reinforcing steel for culverts under 0 to 2 foot fills shall be epoxy-coated.

At the Contractor's option, BV1 & BV2 bars may be spliced at the permissible construction joint in order to facilitate construction. No additional compensation shall be provided for the increase in reinforcing steel quantity due to the splices.

Bar HW2 shall be 4" less than culvert height in length.

Headwall quantities shown assume wingwalls are to be built at a 45° angle to the headwall.

The designs are applicable to the fill height and other conditions indicated. Any change in the conditions invalidates these designs.

Wingwalls referenced by letter apply when the acceptable foundation level is the same for both box and wings. If foundation levels are different, the height of the wingwall shall be adjusted by selection of another lettered wingwall of appropriate height. For wingwall details, refer to standard series BCW for the appropriate fill slope.

For details of extending existing boxes, refer to Standard BCE-01.

For modification of details for skewed culverts, see the Skewed Box Details included in the road plans.

This standard shall be used with the BCQ standard series.

SPECIFICATION REFERENCE

QUADRUPLE BOX CULVERTS

STANDARD DETAILS

VIRGINIA DEPARTMENT OF TRANSPORTATION

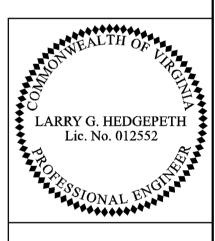
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ROAD AND BRIDGE STANDARDS

REVISION DATE 10/09 SHEET 2 OF 3

1005.02

						REINF	Ę	QUA	N./LF		HEADWALLS								\Box							
SPAN	HGHT						BTI						П	د د			G	Г	HWI			2		⊊	50	11
(FT.)	(FT.)	SIZE	SPACING C-C	a	b	С	d	е	f	g	LENGTH	NO. BL I BARS	NO. BL2 BARS	REINFORCING STEEL (LBS/LONG	CONCRETE CLASS A4 (CY/LF)	REINFORCING STEEL (LBS/LF)	HEADWALL LENGTH	SIZE	LENGTH	NO. HW2 BARS	NO. HW3 BARS	CONCRETE CL. A4 (CY)	INLET REINFORCING STEEL (LBS)	OUTLET CONCRETE CL. A4 (CY)	OUTLET REINFORCING STEEL (LBS)	WINGWALL
3	3	4	12"	2'- 8%	0'- 41/2"	1'- 6%	0'- 5%	0'- 3¾'	14'-11%	1'- 5%	15'- 9"	60	20	57.160	1.284	141.899	16'-10"	6	16'- 6"	3	12	2.222	137.876	2.191	99.132	_
3	4	4	12"	2'- 8%	0'- 41/2"	I'- 6 % '	0'- 5%	0'- 3 ¾ '	14'-11%	1'- 5%	15'- 9"	60	30	60.690	1.406	152.339	16'-10"	6	16'- 6"	3	15	2.333	148.230	2.284	99.132	2
4	3	4	12"	3'- 6 ¹ /4"	0'- 5"	1,-11,	0'- 61/2"	0'- 41/4"	19'- 0"	2'- 1/2"	19'-10"	76	20	70.520	1.539	169,287	20'-10"	6	20'- 6"	3	12	2.606	161.908	2.589	123.164	A
4	4	4	12"	3'- 6 ¹ /4"	0'- 5"	1'-11"	0'- 61/2"	0'- 41/4"	19'- 0"	2'- 1/2"	19'-10"	76	30	74.050	1.661	179,727	20'-10"	6	20'- 6"	3	15	2.718	172.262	2.682	123.164	С
4	5	4	12"	3'- 6 ¹ /4"	0'- 5"	1,-11,	0'- 61/2"	0'- 41/4"	13 0.	2'- 1/2"	19'-10"	76	40	77.580	1.784	190.167	20'-10"	6	20'- 6"	3	18	2.829	182.616	2.774	123.164	E
4	6	4	12"	3'- 6 ¹ /4"	0'- 5"	1,-11,	0'- 61/2"	0'- 41/4"	13 0.	2'- 1/2"	19'-10"	76	50	81.110	1.907	200.607	5010.	6	20'- 6"	3	21	2.940	192.970	2.867	123.164	G
5	3	4	12"	4'- 3"	0'- 6¾"	2'- 2"	0'- 9"	0'- 6"	23'- 0"	2'- 6"	24'- 3"	92	20	83.880	1.998	205.949	24'-10"	6	24'- 6"	3	12	2.932	185.940	2.927	147.196	В
5	4	4	12"	4'- 31/8"	0'- 61/2"	2'- 21/4"	0'- 8%	0'- 5 ¾	23'- 0"	2'- 61/4"	24'- 2"	92	30	87.410	2.085	215.832	24'-10"	6	24'- 6"	3	15	3.056	196.294	3.033	147.196	D
5	5	4	12"	4'- 3"	0'- 6¾	2'- 2"	0'- 9"	0'- 6"	23'- 0"	2'- 6"	24'- 3"	92	40	90.940	2.243	226.829	24'-10"	6	24'- 6"	3	18	3.154	206.648	3.113	147.196	, F
5	6	4	12"	4'- 31/8"	0'- 61/2"	2'- 21/4"	0'- 8%	0'- 5¾	23'- 0"	2'- 61/4"	24'- 2"	92	50	94.470	2.329	242.135	24'-10"	6	24'- 6"	3	21	3.279	217.002	3.219	147.196	Н
5	7	4	12"	4'- 31/8"	0'- 61/2"	2'- 21/4"	0'- 8%	0'- 5¾	23'- 0"	2'- 6 ¹ /4"	24'- 2"	92	60	98.000	2.452	252.575	24'-10"	6	24'- 6"	3	24	3.390	227.356	3.311	147.196	, J
6	4	4	10"	4'-11¾'	0'- 81/2"	2'- 5"	0'-111/2"	0'- 7 ¾	27'- 0"	2'-111/2"	28'- 7"	108	30	100.770	2.665	262.804	28'-10"	6	28'- 6"	3	15	3.307	220.326	3.297	171.228	D
6	5	4	10"	4'-11%"	0'- 8¾	2'- 4¾*	0'-11%	0'- 8"	27'- 0"	2'-111/4"	28'- 8"	108	40	104.300	2.830	274.090	28'-10"	6	28'- 6"	3	18	3.402	230.680	3.373	171.228	F
6	6	4	10-	5'- 0"	0 8.	2'- 51/2"	0'-10¾"	0'- 71/4"	27'- 0"	3'- 0"	28'- 6"	108	50	107.830	2.826	283.640	28'-10"	6	28'- 6"	3	21	3.562	241.034	3.515	171.228	. Н
6	7	4	10"	5'- 0"	0 8.	2'- 51/2"	0'-10%	0'- 71/4"	27'- 0"	3'- 0"	28'- 6"	108	60	111.360	2.948	303.672	28'-10"	6	28'- 6"	3	24	3.673	251.388	3.607	171.228	J
6	8	4	10-	4'-11%	0 81/4.	2'- 51/4"	011/8.	0'- 71/2	27'- 0"	2'-11%	28'- 6"	108	70	114.890	3.112	324.650	28'-10"	6	28'- 6"	3	27	3.768	261.742	3.684	171.228	<u> </u>
7	4	4	9"	5'- 8¾'	0'- 9¾'	2'- 8%	1 1/4.	0'- 9"	31'- 1/8"	3'- 5%	32'-10"	124	30	114.130	3.200	313.216	32'-10"	6	32'- 6"	3	_	3.565	244.358	3.569	195.260	—
7	6	4	9"	5'- 8¾'	0 81/4.	2'- 85%	1 1/4.	0'- 9"	31'- 1/8"	3'- 5%	32'-10"	124	50	121.190	3.443	365.987	32'-10"	6	32'- 6"	3	_	3.788	265.066	3.754	195.260	Щ
7	8	4	9.	5'- 8%	0 3¾.	2'- 8%	1 1/4.	0 3.	31 1/8.	3'- 5%	32'-10"	124	70	128,250	3.687	403.333	32'-10"	6	32'- 6"	3	27	4.011	285.774	3.940	195.260	—
7	10	4	9"	5'- 8¾'	0 8¾.	2'- 8%	1 1/4.	0 8.	31'- 1/8"	3'- 5¾'	32'-10"	124	90	135.310	3.929	459.978	32'-10"	6	32'- 6"	3	_	4.233	306.482	4.125	195.260	
8	4	4	9"	6'- 5%	0'-111/2"	2'-11%"	1 - 3%	0103/4	34'-11%	3'-10%	37'- 3"	140	30	127.490	3.901	416.231	36'-10"	6	36'- 6"	3	_	3.749	268.390	3.765	219.292	—
8	6	4	9"	6'- 5 %	0'-111/2"	2'-11%"	1'- 3¾	0103/4.	34'-11%	3'-10%	37'- 3"	140	50	134.550	4.146	438.001	36'-10"	6	36'- 6"	3	21	3,971	289.098	3.951	219.292	
8	8	4	9"	6'- 51/4"	<u> </u>	- "	1'- 41/8"	011.	34'-11%	3'-10%	37'- 3"	140	70	141.610	4,444	476.814	36'-10"	6	36'- 6"	3	27	4,190	309.806	4.133	219,292	_
8	10	4	9"	6'- 5%	0'-111/2"	2'-11%"	1'- 3¾*	0'-10%	34'-11%"	3'-10%	37'- 3"	140	90	148.670	4.632	534.161	36'-10"	6	36'- 6"	3	33	4,417	330.514	4.322	219.292	—
9	4	5	9.	7'- 2"	1'- 1%	3'- 2 ¹ /2"	1'- 6%	1'- ¾*	39'- 0"	4'- 4"	41'- 7"	156	30	140.850	4.747	544.856	40'-10"	6	40'- 6"	3	_	3.882	292.422	3.912	243.324	—
9	6	5	9"	7'- 2"	1'- 1%	3'- 21/2"	I'- 6 %	1'- ¾"	39'- 0"	4'- 4"	41'- 7"	156	50	147.910	4.992	566.626	40'-10"	6	40'- 6"	3	_	4.104	313.130	4.097	243.324	—
9	8	5	9"	7'- 2"	1'-1%	3'- 2 ¹ /2"	1'- 6%	I'- ¾*	39'- 0"	4'- 4"	41'- 7"	156	70	154.970	5.236	610.906	40'-10"	6	40'- 6"	3	27	4.327	333.838	4.283	243.324	—
9	10	5	9"	7'- 2"	1'- 1%	3'- 2'/2"	1'- 6%	I'- ¾*	39'- 0"	4'- 4"	41'- 7"	156	90	162.030	5.481	631.448	40'-10"	6	40'- 6"	3	_	4.550	354.546	-	243.324	—
9	12	5	9.	7'- 2 ¹ /2'	1'- 15/8"		1'- 6%	1'- ¾	39'- 1"	4'- 4"	41'- 8"	156	110	169.090	5.768	713.655	40'-10"	6	40'- 6"	3	39	4,727	375.254	4.608	243.324	—
10	4	6	9.	7'-10%	1'- 3'/4"	3'- 5¾	1 - 8%	1'- 21/4"	43'- 0"	4 - 9¾	45'-11"	172	30	154.210	5.559	655.664	44'-10"	6	44'- 6"	3	15	4.004	316.454	4.047	267.356	
10	6	5	9"	7'-10%			1'- 8¾	1'- 2'/4	43'- 0"	4'- 9¾	45'-11"	172	-	161.270	5.805	675.010	44'-10"	6	44'- 6"	3	_	4.227	337.162	4.233	267.356	—
10	8	6	9"	7'-10¾"	1'- 31/2"	3'- 51/2"	1'- 91/4"	1'- 21/2"	43'- 0"	4'- 91/2"	45'-11"	172	70	168.330	6.115	718.045	44'-10"	6	44'- 6"	3	27	4,421	357.870	4.390	267.356	
10	10	6	9"	7'-10¾*	1'- 31/2"	3'- 51/2"	1'- 91/4"	1'- 21/2"	43'- 0"	4'- 91/2"	45'-11"	172	90	175.390	6.358	779.808	44'-10"	6	44'- 6"	3		4.643	378.578	4.575	267.356	—
10	12	5	9"	7'-11'/4"	1'- 3%	3'- 51/2"	1'- 91/8"	1'- 21/2"	43'- "	4'- 9/2	46'- 1"	172	110	182.450	6.645	862.627	44'-10"	6	44'- 6"	3	_	4.820	399.286	4,714	267.356	—
12	6	6	8-	9'- 41/2"	1'- 61/2"	4'- 1/8"	2'- 11/2"	1'- 51/2"	51'- 1/8"	5'- 8%	54'- 7"	204	50	187,990	7.729	935.948	52'-10"	6	52'- 6"	3	_	4.344	385.226		315.420	
12	8	6	8.	9'- 41/2"	1'- 6'/2"	-	2'- 11/2"	1'- 5'/2"	51'- 1/8"	5'- 8%	54'- 7"	204	70	195.050	7.972	983.064	52'-10"	6	52'- 6"	3	27	4.567	405.934	-	315.420	—
12	10	6	8-	9'- 41/2"	1'- 61/2"	4'- 1/8"	2'- 11/2"	1'- 51/2"	51'- 1/8"	5'- 8%	54'- 7"	204	90	202.110	8.214	1,050.040	52'-10"	6	52'- 6"	3	33	4.789	426.642	4.748	315.420	—
12	12	6	8"	9'- 5"	1'- 61/2"	4'- 1/8"	2'- 11/2"	1'- 51/2"	51'- 11/8"	5'- 8%	54'- 8"	204	110	209.170	8.504	1,114.396	52'-10"	6	52'- 6"	3	39	4.964	447.350	4.885	315.420	V



VDOT S&B Division Richmond, Virginia Structural Engineer

VIRGINIA DEPARTMENT OF TRANSPORTATION

QUADRUPLE BOX CULVERTS

25 TO 30 FT. FILLS

STANDARD	REVISION DATE	SPECIFICATION REFERENCE	SHEET 2 OF 2
BCQ-30	10/09		1005.17

Specifications:

Construction - Va. Department of Transportation Road and Bridge Specifications, 2007. Design - AASHTO Standard Specifications for Highway Bridges, 1983, Including Interim Specifications, 1984, 1985 and VDOT Modifications, using Load Factor Design.

All concrete shall be Class A4.

Deformed reinforcing bars shall conform to ASTM A615, Grade 60. All reinforcing bar dimensions on the detailed drawing are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

Dimensions on bar diagrams are out-to-out of bars. Bars are straight unless otherwise shown.

The centers of main reinforcing bars shall be 2" from the face of the concrete unless otherwise shown.

At the Contractor's option, WV Series bars may be spliced at the top of footing in order to facilitate construction. Splice lengths shall be in accordance with TABLE C. No additional compensation shall be provided for the increase in reinforcing steel quantity due to the splices.

When concrete protective coating is required, all steel shall be epoxy-coated.

Bearing capacity of foundations shall be 1.5 Tons/Sq. Ft. minimum for wings A - X and 2 Tons/Sq. Ft. minimum for wings Y - EE.

Weepholes shall be placed at lowest point feasible for free drainage away from wing.

Four Type I Wings are to be used for straight crossings and skews up to 20°. Two Type I & two Type II Wings are to be used for skews from 25° to 45°. For skews above 45°, special design wings are required. The wingwall to be used for each culvert is shown on the BC series sheets.

The designs shown are applicable for a 45° skew with the roadway and other conditions indicated. Any change in these conditions invalidates these designs.

Quantities shown are for one wing.

	Quantity (One Wing)		
	Concrete	Reinforcing Steel	
WING	Cu. Yd.	Lbs.	
A	1.692	102.747	
В	2.086	134.045	
С	2.498	158.678	
D	2.627	171.704	
Ε	3.187	202.147	
F	3.669	225.269	
G	4.084	256.122	
Н	4.630	287.351	
Ι	5.395	345.119	
J	6.163	386.348	
К	7.093	447.595	
L	7.394	485.032	
М	8.090	549.490	
N	8.735	601.075	
0	9.496	662.513	
Р	10.480	972.915	
a	10.995	1009.521	
R	12.071	1141.937	
s	12.962	1028.293	
Т	16.931	1149.082	
U	17.605	1533.982	
٧	19.070	1841.681	
w	20.946	2596.967	
х	22.546	2009.473	
Y	24.453	1968.517	
Z	26.215	2385.838	
AA	27.508	3913.960	
BB	28.542	2710.731	
СС	29.932	3799.925	
DD	36.104	4040.276	
EE	37.813	4568.249	

SPECIFICATION REFERENCE	WING DETAILS
	1/2:1 FILL SLOPE - TYPE I
	VIRGINIA DEPARTMENT OF TRANSPORTATION

₩DOTROAD AND BRIDGE STANDARDS

REVISION DATE 10/09

Specifications:

Construction - Va. Department of Transportation Road and Bridge Specifications, 2007.

Design - AASHTO Standard Specifications for Highway Bridges, 1983, Including Interim Specifications, 1984, 1985 and VDOT Modifications, using Load Factor Design.

All concrete shall be Class A4.

Deformed reinforcing bars shall conform to ASTM A615, Grade 60. All reinforcing bar dimensions on the detailed drawing are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

Dimensions on bar diagrams are out-to-out of bars. Bars are straight unless otherwise shown.

The centers of main reinforcing bars shall be 2" from the face of the concrete unless otherwise shown.

At the Contractor's option, WV Series bars may be spliced at the top of footing in order to facilitate construction. Splice lengths shall be in accordance with TABLE C. No additional compensation shall be provided for the increase in reinforcing steel quantity due to the splices.

When concrete protective coating is required, all steel shall be epoxy-coated.

Bearing capacity of foundations shall be 1.5 Tons/Sq. Ft. minimum for wings A - X and 2 Tons/Sq. Ft. minimum for wings Y - EE.

Weepholes shall be placed at lowest point feasible for free drainage away from wing.

Four Type I Wings are to be used for straight crossings and skews up to 20°. Two Type I & two Type II Wings are to be used for skews from 25° to 45°. For skews above 45° , special design wings are required. The wingwall to be used for each culvert is shown on the BC series sheets.

The designs shown are applicable for a 45° skew with the roadway and other conditions indicated. Any change in these conditions invalidates these designs.

Quantities shown are for one wing.

	Quantity (One Wing)
	Concrete	Reinforcing Steel
WING	Cu. Yd.	Lbs.
A	2.110	133.183
В	2.504	165.003
С	2.916	188.084
D	3.347	211.979
Ε	3.796	248.635
F	4.424	276.524
G	4.923	318.386
Н	5.836	360.316
- 1	6.392	388.442
J	7.186	454.546
K	8.026	511.375
L	8.913	572.281
М	9.582	626.003
N	10.546	702.040
0	11.825	879.295
Р	12.580	964.846
G	13.679	1063.138
R	14.810	1350.012
S	17.199	1225.256
Т	19.817	1532.432
U	21.317	1745.661
٧	22.858	2152.605
W	23.964	1823.343
x	25.975	2817.287
Y	28.546	2310.426
Z	30.359	3017.503
AA	32.262	3328.030
BB	34.212	3818.744
CC	35.571	4104.313
DD	37.575	5249.194
EE	40.334	4171.222

PECIFICATION REFERENCE	WING DETAILS
	I 1/2: I FILL SLOPE - TYPE Ⅱ
	VIRGINIA DEPARTMENT OF TRANSPORTATION

VDOT

ROAD AND BRIDGE STANDARDS

REVISION DATE

Specifications:

Construction - Va. Department of Transportation Road and Bridge Specifications, 2007.

Design - AASHTO Standard Specifications for Highway Bridges, 1983, including Interim Specifications, 1984, 1985 and VDOT Modifications, using Load Factor Design.

All concrete shall be Class A4.

Deformed reinforcing bars shall conform to ASTM A615, Grade 60. All reinforcing bar dimensions on the detailed drawing are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

Dimensions on bar diagrams are out-to-out of bars. Bars are straight unless otherwise shown.

The centers of main reinforcing bars shall be 2" from the face of the concrete unless otherwise shown.

At the Contractor's option, WV Series bars may be spliced at the top of footing in order to facilitate construction. Splice lengths shall be in accordance with TABLE C. No additional compensation shall be provided for the increase in reinforcing steel quantity due to the splices.

When concrete protective coating is required, all steel shall be epoxycoated.

Bearing capacity of foundations shall be 1.5 Tans/Sq. Ft. minimum for wings A - X and 2 Tans/Sq. Ft. minimum for wings Y - EE.

Weepholes shall be placed at lowest point feasible for free drainage away from wing.

Four Type I Wings are to be used for straight crossings and skews up to 20°. Two Type I & two Type II Wings are to be used for skews from 25° to 45°. For skews above 45°, special design wings are required. The wingwall to be used for each culvert is shown on the BC series sheets.

The designs shown are applicable for a 45° skew with the roadway and other conditions indicated. Any change in these conditions invalidates these designs.

Quantities shown are for one wing.

	Quantity (One Wing)
	Concrete	Reinforcing Steel
WING	Cu. Yd.	Lbs.
A	2.334	146.814
В	2.741	179.226
С	3.208	202.300
D	3.658	227.238
Ε	4,174	268.139
F	4.665	294.699
G	5.412	348.564
Н	6.153	378.666
ı	6.798	431.368
J	7.621	479.805
К	8.567	531.895
L	9.236	601.618
М	10.275	652.010
N	10.623	706.389
0	11.644	765.083
Р	12.804	868.664
٥	13.615	938.923
R	14.852	1247.994
S	16.063	1193.231
Т	17.077	1263.468
U	18.343	1566.661
٧	19.708	2817.906
w	21.169	1838.465
×	24.710	2352.406
Y	29.398	2856.703
Z	31.458	2680.408
AA	33.355	3557.475
BB	36.175	3205.508
СС	38.270	3362.599
DD	45.675	4925.833
EE	48.208	5122.544

Augostitu (Ann Wing)

SPECIFICATION REFERENCE	WING DETAIL
	2: 1 FILL SLOPE - TYPE I
	VIRGINIA DEPARTMENT OF TRANSPORTATION

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ROAD AND BRIDGE STANDARDS

REVISION DATE 10/09

Specifications:

Construction - Va. Department of Transportation Road and Bridge Specifications, 2007.

Design - AASHTO Standard Specifications for Highway Bridges, 1983, Including Interim Specifications, 1984, 1985 and VDOT Modifications, using Load Factor Design.

All concrete shall be Class A4.

Deformed reinforcing bars shall conform to ASTM A615, Grade 60. All reinforcing bar dimensions on the detailed drawing are to centers of bars except where otherwise noted and are subject to fabrication and construction tolerances.

Dimensions on bar diagrams are out-to-out of bars. Bars are straight unless otherwise shown.

The centers of main reinforcing bars shall be 2" from the face of the concrete unless otherwise shown.

At the Contractor's option, WV Series bars may be spliced at the top of footing in order to facilitate construction. Splice lengths shall be in accordance with TABLE C. No additional compensation shall be provided for the increase in reinforcing steel quantity due to the splices.

When concrete protective coating is required, all steel shall be epoxy-coated.

Bearing capacity of foundations shall be 1.5 Tons/Sq. Ft. minimum for wings A - X and 2 Tons/Sq. Ft. minimum for wings Y - EE.

Weepholes shall be placed at lowest point feasible for free drainage away from wing.

Four Type I Wings are to be used for straight crossings and skews up to 20. Two Type I & two Type II Wings are to be used for skews from 25 to 45. For skews above 45, special design wings are required. The wingwall to be used for each culvert is shown on the BC series sheets.

The designs shown are applicable for a 45° skew with the roadway and other conditions indicated. Any change in these conditions invalidates these designs.

Quantities shown are for one wing.

	Quantity (One Wing)	
	Concrete	Reinforcing Steel
WING	Cu. Yd.	Lbs.
A	2.917	182.454
В	3.499	233.188
С	3.939	252.894
D	4.573	286.711
E	5.233	341.981
F	6.145	383.724
G	6.881	445.681
Н	7.910	489.811
Ι	8.512	517.004
J	9.643	613.342
К	10.529	655.405
L	11.777	771.234
М	13.099	837.992
N	14.106	919.335
0	15.275	1017.802
Р	16.355	1093.189
Q	17.891	1292.357
R	19.035	1525.633
S	20.691	1704.239
Т	21.910	1952.852
u	23.368	2116.654
٧	24.678	2112.153
W	28.412	2270.223
x	32.434	3904.347
Y	35.972	2730.572
Z	37.707	2849.343
AA	39.857	3392.164
88	42.547	3594.334
СС	46.155	4755.614
DD	49.940	5500.464
EE	51.922	6808.705

SPECIFICATION REFERENCE	WING DETAILS
	2:1 FILL SLOPE - TYPE II
	VIRGINIA DEPARTMENT OF TRANSPORTATION

₩DOTROAD AND BRIDGE STANDARDS

REVISION DATE