Appendix 12B-3 LD-293 Hydraulic Analysis Report

HYDROLOGIC AND HYDRAULIC ANALYSIS REPORT

OCATION			
LOCATION			
Project	:		
Route County/City	:		
County/City	:		
Waterway	:		
PREPARED BY			
Name Organization	:		
Date			
	•		
STRUCTURE D	ESCRIPTIO	ON	
Abutment A Station	on: Fin	ished Grade Elevation	ft. (m)
		ished Grade Elevation	ft. (m)
		on ft. (m)	
	centerline	to flood flow	
Span Length			
Abutment Type			
Number/Type Pie	ers		
HYDROLOGIC	/HVDD AIII	IC DATA	
IIIDKOLOGIC	/III DRAUL	EIC DATA	
Orainage Area		_ Sq. Mi. (km ²)	
HISTORICAL D	ЭΑΤΑ		
High Water Eleva	ation	ft. (m) Date of Occurrence	
Estimated Dischar	rge	cfs. (m^3/s)	
Estimated Exceed	lence Probab	ility %	

HYDRAULIC PERFORMANCE

The data presented herein is the result of statistical analysis and indicates an approximate estimate of the performance of this facility.

Discharge	Estimated	Change in existing	Flood stage	Velocity thru
(cfs)(m ³ /s)	Exceedence Probability (%)	flood levels (ft.)(m)	upstream of bridge (ft.)(m)	Bridge Structure (ft/s)(m/s)
	50%			
	20%			
	10%			
	4%			
	2%			
	1% Natural			
	1% Floodway			
	0.2%			

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DESIGN SUMMARY

	Exceedence Probability (%)	Stage Elevation (ft.)(m)	
Design Flood			
Overtopping Flood			
Base Flood			
Ordinary High Water			

DEBRIS POTENTIAL

ABUTMENT SLOPE PROTECTION RECOMMENDATIONS

26" Class I Dry Riprap over 4" no. 25 or 26 aggregate over filter cloth will be hydraulically satisfactory.
650 mm Class I Dry Riprap over 100 mm no. 25 or 26 aggregate over filter cloth will be hydraulically satisfactory.
950 mm Class II Dry Riprap over 150 mm no. 25 or 26 aggregate over filter cloth will be hydraulically satisfactory.
38" Class II Dry Riprap over 6" no. 25 or 26 aggregate over filter cloth will be hydraulically satisfactory.

SCOUR PLOTS

A sketch of the final scoured bed profile and the check scoured bed profile is attached. If scour countermeasures are required, a request must be submitted to the Hydraulics Unit for their design and documentation.

CAUSEWAYS

The use of causeways for temporary construction access was not considered in this analysis. If it is subsequently found necessary to use causeways, they must be submitted to the Hydraulics Unit for analysis and documentation.

Temporary construction access causeways for this project should be composed of < <specify>>.</specify>
The ordinary highwater will be increased by ft. (m)
The high flow profiles will not be affected.
The causeway will not affect the water surface profile.
The maximum causeway elevation is ft. (m)
From abutment A to station
From station to abutment B.
Only one will be in place at a time.

STREAM BANK STABILIZATION

The banks should reestablish themselves to the natural conditions.

The Riprap should be placed on all areas that will not support vegetation.

Disturbed areas outside the bridge should be seeded.

COMMENTS

Note any channel modifications, flood plain impacts and impact mitigation measures as well as other data pertinent to the design. Also comment on the feasibility of using a smaller structure.

This analysis is only applicable to the structures(s) and approaches described. Any changes in these conditions may invalidate this analysis and should be reviewed by this office.

This design represents the smallest structure practicable for use at this site.

Appendix 12B-3 LD-293 Hydraulic Analysis Report The existing structure and the existing approach roadways from station: to station: are to be removed and the land is

to be regraded to its natural contour.	are to be removed	and the fand is
If this project is an interstate or other NHS project and is expected to be in excess the FHWA that (1) no hydraulic impacts are anticipated or (2) the following hydraulic impacts are anticipated or (2) the following hydraulic impacts are anticipated or (3) the following hydraulic impacts are anticipated or (1) the following hydraulic impacts are anticipated or (2) the following hydraulic impacts are anticipated or (3) the following hydraulic impacts are anticipated or (1) the following hydraulic impacts are anticipated or (2) the following hydraulic impacts are anticipated or (2) the following hydraulic impacts are anticipated or (3) the following hydraulic impacts are anticipated or (4) the following hydraulic impacts are anticipated or (5) the following hydraulic impacts are anticipated or (6) the following hydraulic impacts are anticipated or (8) the following hydraulic		
If you have any questions or need additional information, please contact via electronic mail at The completed "CONFIRMATION sent to	atats	or hould also be

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HYDROLOGIC DATA SHEET

The information presented hereon is to be transcribed to the Hydrologic Data sheet contained in the plan assembly.

LOCATION	
Project	:
Route	:
County/City	:
Waterway	:
DESCRIPTION	
Sheet No	Station
	2
Drainage Area	sq. mi (km ²)
Structure Size	
BASE FLOOD	
Discharge	cfs (m ³ /s)
<u> </u>	
Stage Elevation _	ft. (m)
DESIGN FLOO	D
Discharge	cfs (m ³ /s)
Estimated Exceed	dence Probability%
Stage Elevation	ft. (m)
Stage Elevation_	n. (m)
OVERTOPPING	G FLOOD
Stage Elevation	ft. (m)
Estimated Exceed	lence Probability %
	, <u> </u>
HISTORICAL I Date	DATA
Stage Elevation _	ft. (m)
Estimated Exceed	dence Probability%
REMARKS	

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CONFIRMATION OF DESIGN

The bridge designer will complete this form and forward it to the Hydraulics Unit confirming that the design that was analyzed is being used.

LOCATION			
Project	:		
Route	:		
County/City	:		
Waterway	:		
STRUCTURE	DESCRIPTION		
Abutment A Station:		Finished Grade Elevation	ft. (m)
Abutment B Station:		Finished Grade Elevation	ft. (m)
Minimum Low	Chord Elevation	ft. (m)	
Skew	_to centerline	to flood flow	
Span Length			
Abutment Type	e		
Number/Type 1	Piers		